

SPREADER BAR

(MAGNET  
COIL ASSEMBLY  
FIXTURE)

I.D. N° 23

COLOR OF BAR :

WHITE

LOAD CAPACITY PAINTED  
ON BAR 4 TONS.

DATE CAP. & I.D. N° PAINTED  
ON BAR \_\_\_\_\_

DATE OF LAST LOAD

TEST. OCTOBER 31 . 88 N. J. Boscik

TEST LOAD WEIGHT 6 TONS

TEST LOAD % 150

STRESS CALCULATIONS :

DONE BY ANDREW SZYMILANSKI

DATE OCT. 31. 88

REMARKS :

MAGNET  
ASSEMBLY FIXTURE -  
LOAD TEST ARRANGEMENT

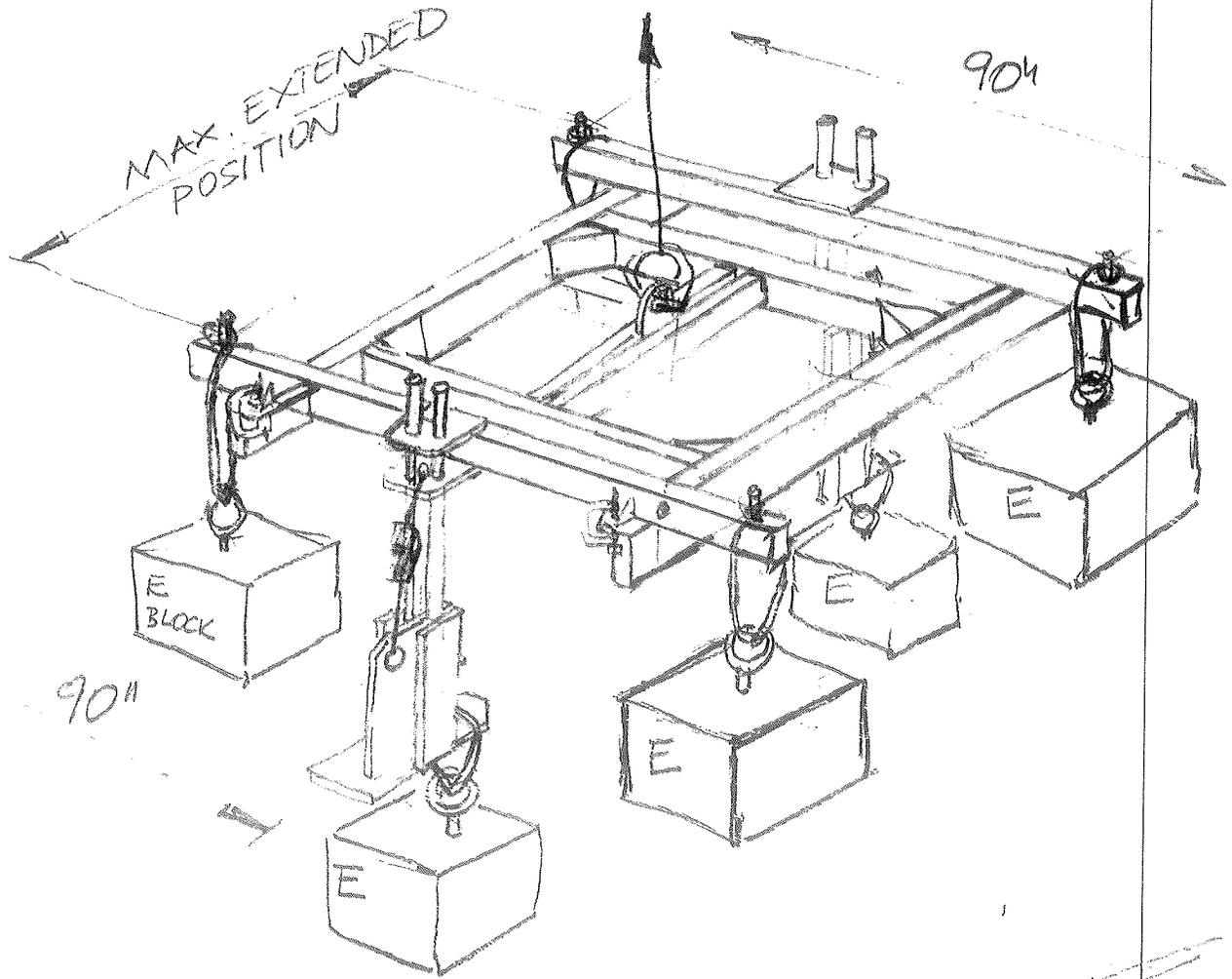


FIG. 1

LOAD: (6) "E" SIZE BLOCKS

MAGNET ASSEMBLY FIXTURE -

X487C

4 POINT PICKUP - MODIFICATION

SPREADER BAR. ID No 23  
(SUPPLEMENTAL TEST)

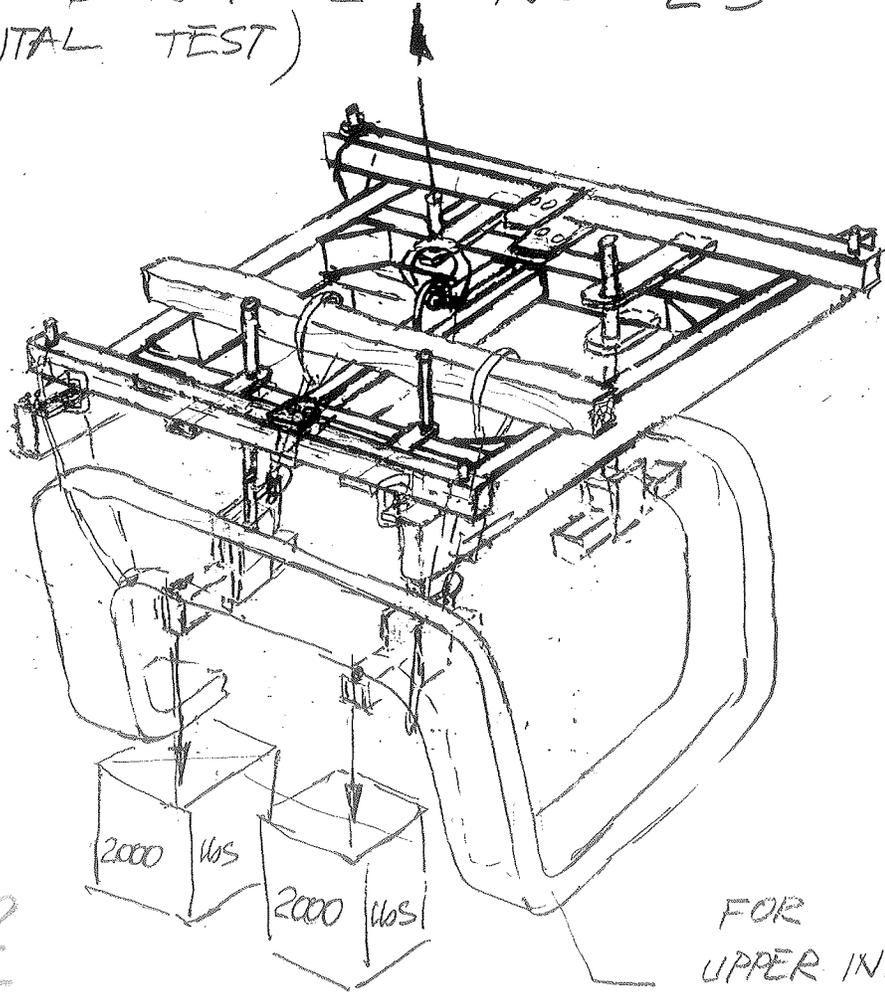


FIGURE 2

LOAD TEST :

FOR UPPER INSIDE 2 COILS ONLY

2000 lbs AT 4 POINTS

9 1/2" EXTENSION AS SHOWN ON

ATTACHED DNG. : "ASSEMBLY FIXTURE - 4 POINT LIFTING FOR UPPER TWO INNER COILS"

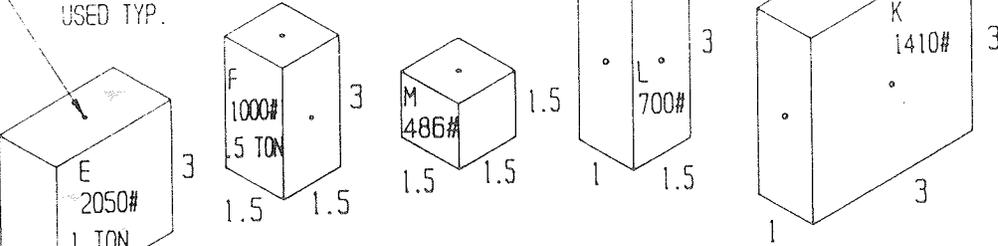
# CONCRETE SHIELDING

DIMENSIONS & WEIGHTS

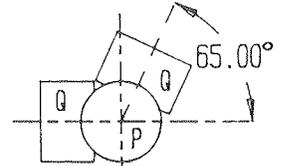
REV. 1988

## SMALL BLOCKS

1 1/4" LIFTING EYE BOLT  
USED TYP.

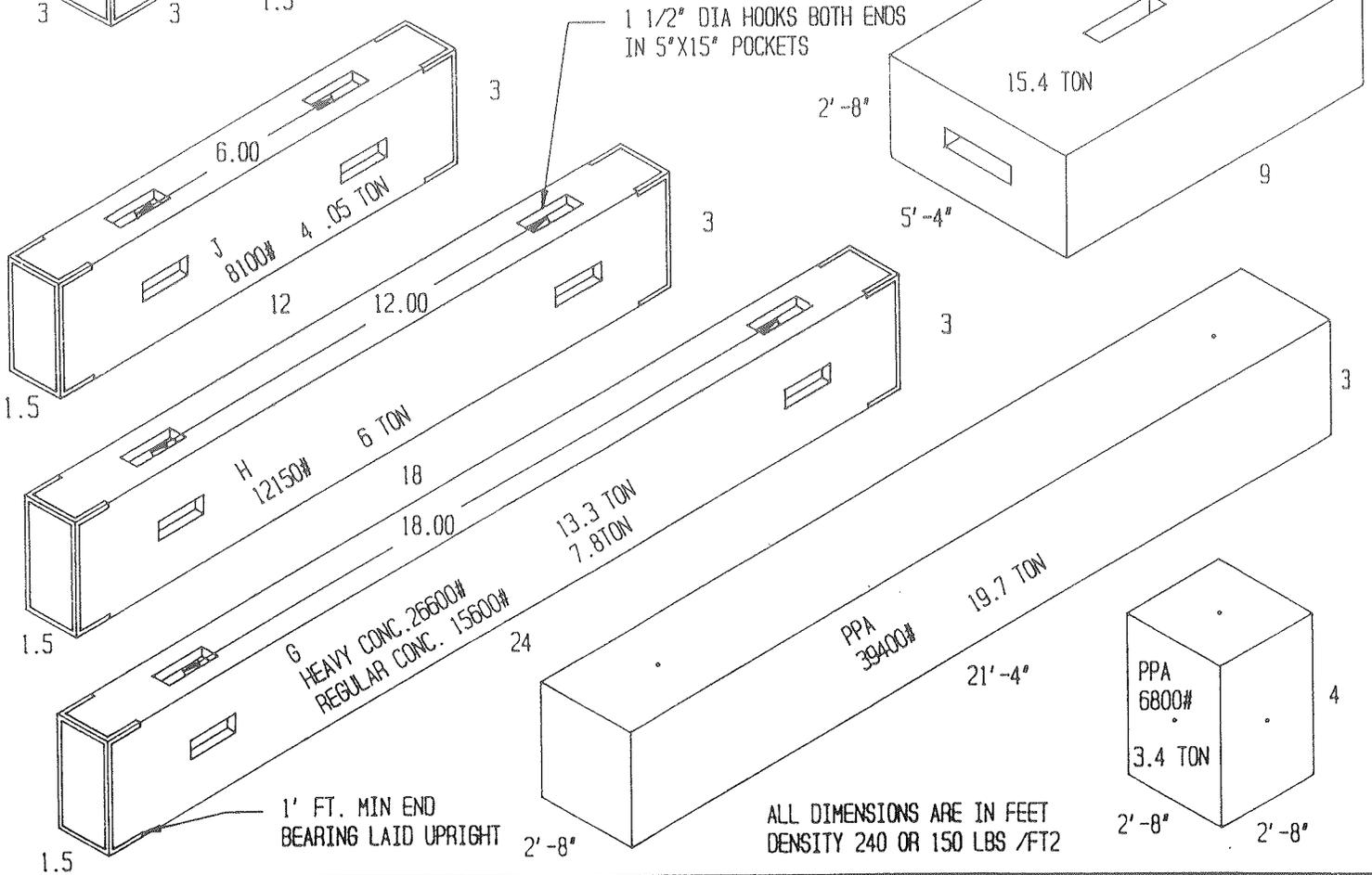
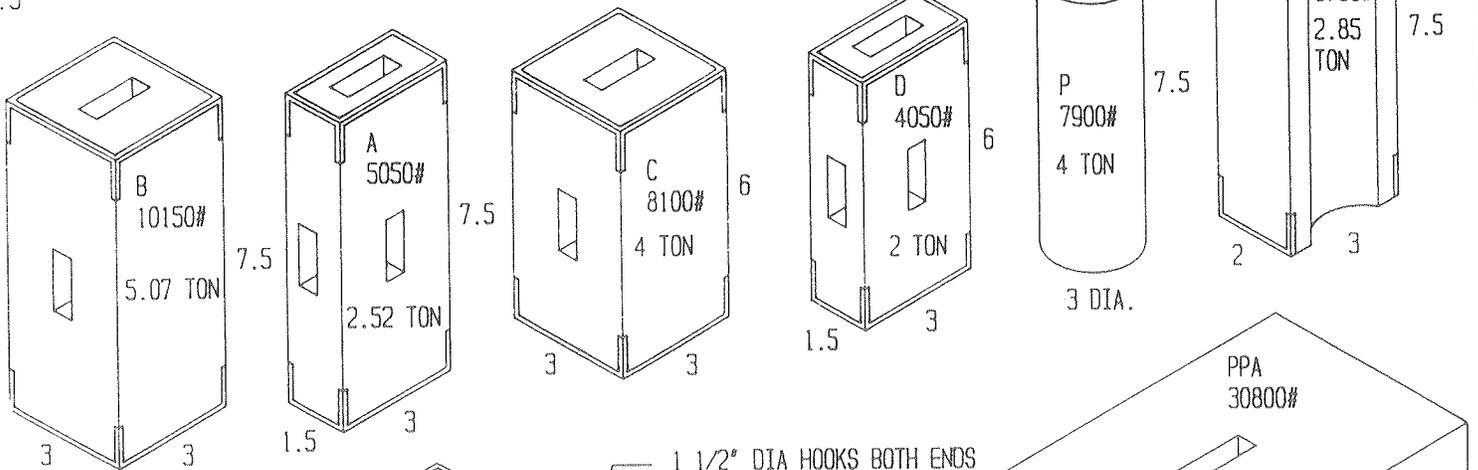


## HINGE BLOCKS



PLAN VIEW SHOWING USAGE  
OF HINGE BLOCKS P & Q

## LARGE BLOCKS



ALL DIMENSIONS ARE IN FEET  
DENSITY 240 OR 150 LBS /FT<sup>2</sup>

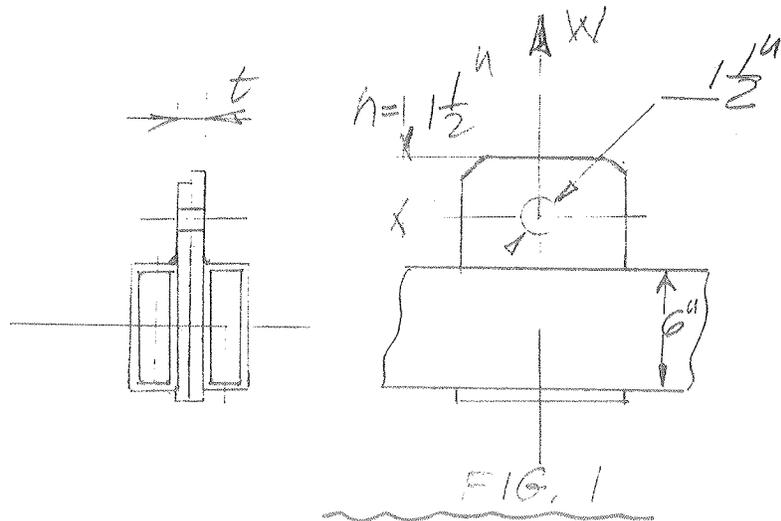
THE FIXTURE IS SHOWN ON THE FOLLOWING DRAWINGS:

1. 9210-350-ME-272068
2. 9210-350-ME-272106
3. 9210-350-ME-272107

DWG. ME-272068

DETERMINING THE CRUCIAL STRESSES IN STRUCTURE ELEMENTS.

ITEMS 7, 8.



$$W = 1.5 \times \text{LOAD} = 12000 \text{ [lbs]}$$

STRESS:

$$S = \frac{W}{h(t)}$$

$$S = \frac{12000}{1.5(1)}$$

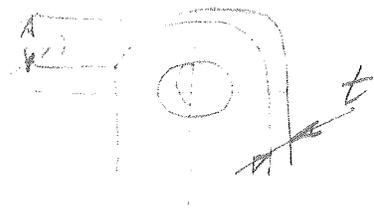
$$S = 8000 \text{ psi}$$

$$S < 0.6 \sigma_y$$

$$8000 < 0.6(36000)$$

$$8000 < 21600$$

O.K.



WELDMENT OF ITEMS 7, 8 TO ITEM 3.

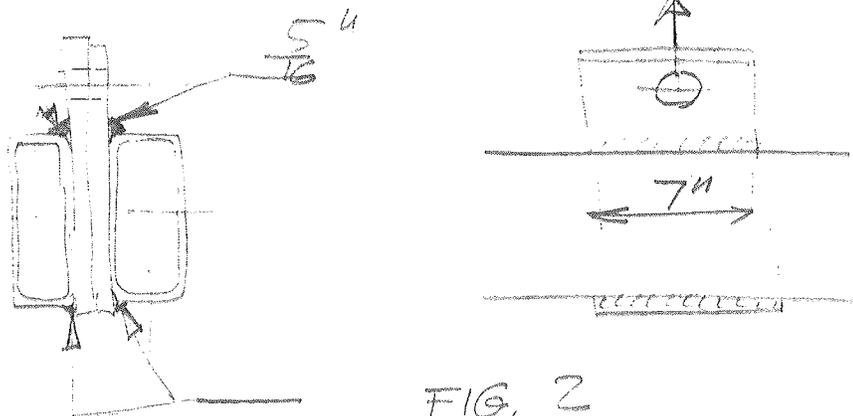


FIG. 2

ALLOWABLE STATIC LOAD PER LINEAR INCH  
ACCORDING TO TABLE 28 (DESIGN OF  
MACHINE MEMBERS)

$$\frac{5}{16} \times \frac{5}{16} \quad \text{FOR NORMAL WELDS ;}$$

$$F = 3125 \times 2 \times 7 = 43750 \text{ lbs.}$$

$$12000 < 43750 \quad \text{O.K.}$$

For weld proportions used in practice, this variation may be neglected. In a parallel weld, the maximum shear stress is across the throat of the weld where the area is  $0.707t$ . Hence the shear stress on the side of the

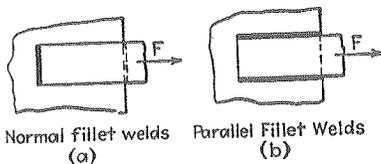


FIG. 88.

weld is  $0.707$  times  $s_s$  MAX, and the permissible load per linear inch is  $0.707t$  times 11,500, or 8,130t lb. This is the same capacity as that found for a normal weld. However, when the external forces are not colinear, there will be bending in the

weld and the permissible load should be reduced 20 per cent to 6,400t. This value has been used in computing the values in Table 28.

TABLE 28.—ALLOWABLE LOADS ON MILD-STEEL FILLET WELDS

Size of weld, in.	Allowable static load per linear inch of weld, lb			
	Bare welding rod		Shielded arc	
	Normal weld	Parallel weld	Normal weld	Parallel weld
$\frac{1}{8}$ by $\frac{1}{8}$	1,000	800	1,250	1,000
$\frac{1}{8}$ by $\frac{1}{4}$	1,500	1,200	1,875	1,500
$\frac{1}{4}$ by $\frac{1}{8}$	2,000	1,600	2,500	2,000
$\frac{1}{4}$ by $\frac{1}{4}$	2,500	2,000	3,125	2,500
$\frac{3}{8}$ by $\frac{1}{8}$	3,000	2,400	3,750	3,000
$\frac{3}{8}$ by $\frac{1}{4}$	4,000	3,200	5,000	4,000
$\frac{3}{8}$ by $\frac{3}{8}$	5,000	4,000	6,250	5,000
$\frac{1}{2}$ by $\frac{1}{8}$	6,000	4,800	7,500	6,000

ALUM  
(1/2 STRONG)

160  
240  
320  
400  
480  
600  
800  
960

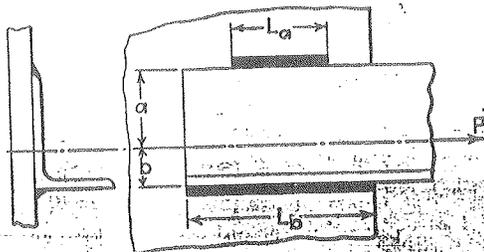
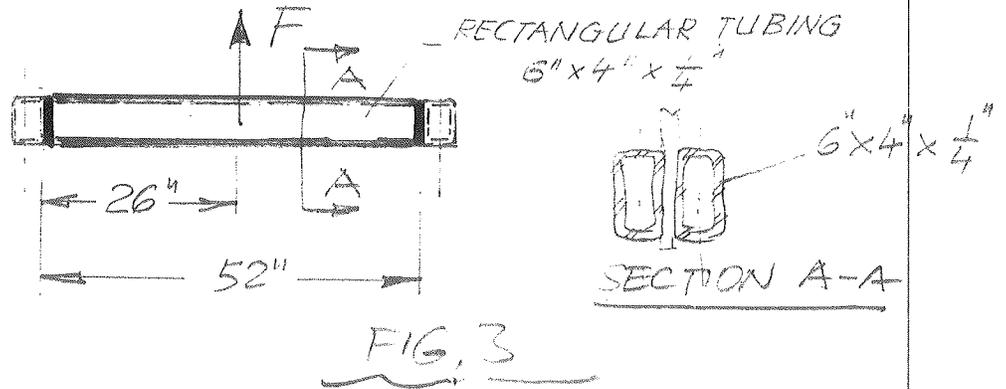


FIG. 89.

138. Eccentric Loads. With axial loads on unsymmetrical sections such as angles, or channels welded on the flange edges, the weld lengths should be proportioned so that the sum of the resisting moments of the welds about the gravity axis is zero. In Fig. 89, let  $L$  be the total weld

ALLOY AND TEMPER
1100 - H12, H14
3003 - H12, H14, H16, H18
ALCLAD 3003 - H12, H14, H16, H18
3004 - H14, H16, H18, H32, H34, H36
ALCLAD 3004 - H14, H16, H18, H291, H32
3105 - H14, H16
5005 - H12, H14, H32, H34
5050 - H32, H34
5052 - H291, H32, H34
5083 - H111
5083 - H321
5083 - H321
5083 - H323, H343
5086 - H111
5086 - H112
5086 - H112
5086 - H112
5086 - H32, H34
5454 - H111
5454 - H112
5454 - H32, H34
5456 - H111
5456 - H112
5456 - H321
5456 - H32
5456 - H112
6061 -
6061 -
6063
6070
76,
AND
H1
9

ITEMS 3;



STRENGTH OF MATERIALS  
Stresses and Deflections in Beams

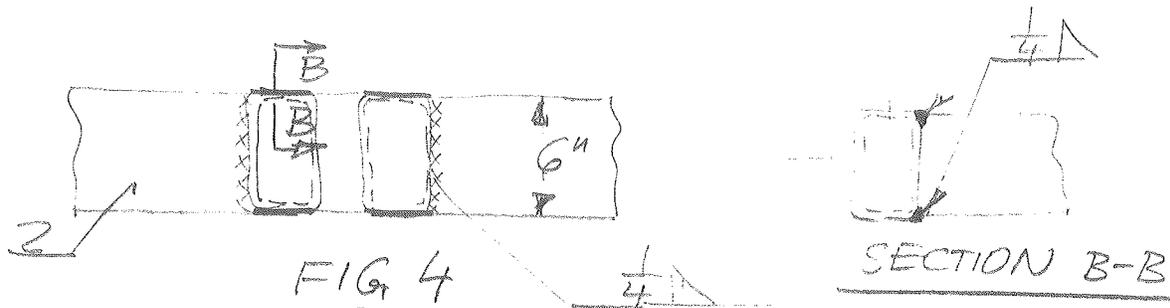
Type of Beam	Stresses	
	General Formula for Stress at any Point	Stresses at Critical Points
Case 19. — Fixed at Both Ends, Load at Center 	Between each end and load, $s = \frac{W}{2Z} (\frac{1}{2}l - z)$	Stress at ends $\frac{Wl}{8Z}$ ; at load $-\frac{Wl}{8Z}$ These are the maximum stresses and are equal and opposite. Stress is zero at $z = \frac{1}{4}l$

MAX. STRESS :

$$S = \frac{Wl}{8Z}$$

$$S = \frac{12000(52)}{8(7.36)(2)} = 5298 \text{ psi}$$

ITEMS 3 AND 2 WELDMENTS :

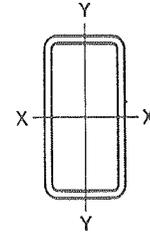


ALLOWABLE STATIC LOAD ACCORDING TO TABLE 28 DATA

NORMAL WELDS :  $F_1 = 2500 \times 8 = 20000$

1 - 95

### STRUCTURAL TUBING Rectangular Dimensions and properties



PROPERTIES**			
Y-Y AXIS			
$r_x$	$I_y$	$S_y$	$r_y$
in.	in. <sup>4</sup>	in. <sup>3</sup>	in.
3.55	80.8	26.9	2.37
3.62	65.4	21.8	2.43
3.65	56.5	18.8	2.46
3.69	46.9	15.6	2.49
3.72	36.5	12.2	2.51
3.31	30.8	15.4	1.58
3.39	25.5	12.8	1.63
3.43	22.4	11.2	1.66
3.47	18.8	9.39	1.69
	14.8	7.39	1.72
3.06	4.85	4.85	.775
3.10	4.42	4.42	.802
3.15	3.85	3.85	.830
3.20	3.14	3.14	.858
2.89	65.7	21.9	2.31
2.96	53.5	17.8	2.36
2.99	46.4	15.5	2.39
3.02	38.6	12.9	2.42
3.05	30.1	10.0	2.45
2.69	24.6	12.3	1.54
2.77	20.6	10.3	1.60
2.80	18.1	9.05	1.62
2.84	15.3	7.63	1.65
2.88	12.0	6.02	1.68

DIMENSIONS				PROPERTIES**						
Nominal* Size	Wall Thickness		Weight per Foot	Area	X-X AXIS			Y-Y AXIS		
	in.	in.			$I_x$	$S_x$	$r_x$	$I_y$	$S_y$	$r_y$
in.	in.	in.	Lb.	in. <sup>2</sup>	in. <sup>4</sup>	in. <sup>3</sup>	in.	in. <sup>4</sup>	in. <sup>3</sup>	in.
8 x 3	.3750	3/8	24.93	7.33	51.0	12.7	2.64	10.4	6.92	1.19
	.3125	5/16	21.21	6.23	44.7	11.2	2.68	9.25	6.16	1.22
	.2500	1/4	17.32	5.09	37.6	9.40	2.72	7.90	5.26	1.25
	.1875	3/16	13.25	3.89	29.6	7.40	2.76	6.31	4.21	1.27
8 x 2	.3750	3/8	22.37	6.58	40.1	10.0	2.47	3.85	3.85	.765
	.3125	5/16	19.08	5.61	35.5	8.87	2.51	3.52	3.52	.792
	.2500	1/4	15.62	4.59	30.1	7.52	2.56	3.08	3.08	.819
	.1875	3/16	11.97	3.52	23.9	5.97	2.60	2.52	2.52	.847
7 x 5	.5000	1/2	35.24	10.4	63.5	18.1	2.48	37.2	14.9	1.90
	.3750	3/8	27.48	8.08	52.2	14.9	2.54	30.8	12.3	1.95
	.3125	5/16	23.34	6.86	45.5	13.0	2.58	26.9	10.8	1.98
	.2500	1/4	19.02	5.59	38.0	10.9	2.61	22.6	9.04	2.01
7 x 4	.5000	1/2	35.24	10.4	63.5	18.1	2.48	37.2	14.9	1.90
	.3750	3/8	27.48	8.08	52.2	14.9	2.54	30.8	12.3	1.95
	.3125	5/16	23.34	6.86	45.5	13.0	2.58	26.9	10.8	1.98
	.2500	1/4	19.02	5.59	38.0	10.9	2.61	22.6	9.04	2.01
7 x 3	.5000	1/2	35.24	10.4	63.5	18.1	2.48	37.2	14.9	1.90
	.3750	3/8	27.48	8.08	52.2	14.9	2.54	30.8	12.3	1.95
	.3125	5/16	23.34	6.86	45.5	13.0	2.58	26.9	10.8	1.98
	.2500	1/4	19.02	5.59	38.0	10.9	2.61	22.6	9.04	2.01
6 x 4	.5000	1/2	28.43	8.36	35.3	11.8	2.06	18.4	9.21	1.48
	.3750	3/8	22.37	6.58	29.7	9.90	2.13	15.6	7.82	1.54
	.3125	5/16	19.08	5.61	26.2	8.72	2.16	13.8	6.92	1.57
	.2500	1/4	15.62	4.59	22.1	7.36	2.19	11.7	5.87	1.60
6 x 3	.5000	1/2	28.43	8.36	35.3	11.8	2.06	18.4	9.21	1.48
	.3750	3/8	22.37	6.58	29.7	9.90	2.13	15.6	7.82	1.54
6 x 2	.5000	1/2	28.43	8.36	35.3	11.8	2.06	18.4	9.21	1.48
	.3750	3/8	22.37	6.58	29.7	9.90	2.13	15.6	7.82	1.54

\* Outside dimensions across flat sides.

\*\* Properties are based upon a nominal outside corner radius equal to two times the wall thickness.

radius equal to two times the

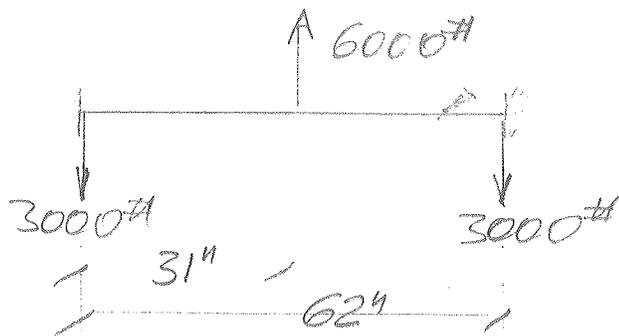
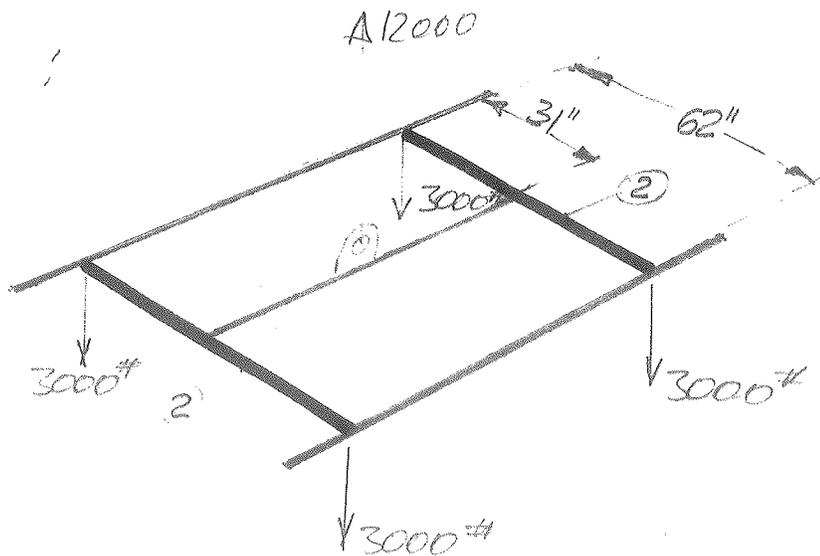
PARALLEL WELDS :

$$F_2 = 2000 \times 4 = 8000$$

$$\text{TOTAL: } F_T = F_1 + F_2 = 28000 \text{ lbs}$$

$$12000 < 28000 \quad \text{O.K.}$$

ITEMS 2 :



6" x 4" x 5/16" TUBING

FIG. 5

Stresses and Deflections in Beams

Type of Beam	Stresses	
	General Formula for Stress at any Point	Stresses at Critical Points
Case 19. — Fixed at Both Ends, Load at Center 	Between each end and load, $s = \frac{W}{2Z} (\frac{1}{2}l - x)$	Stress at ends $\frac{Wl}{8Z}$ ; at load $-\frac{Wl}{8Z}$ These are the maximum stresses and are equal and opposite. Stress is zero at $x = \frac{1}{4}l$

Stresses and Deflections in Beams

Deflections (See footnote at beginning of Table)	
General Formula for Deflections at any Point	Deflections at Critical Points
$y = \frac{Wx^2}{48EI} (3l - 4x)$	Maximum deflection, at load, $\frac{Wl^3}{192EI}$

MAX. STRESS :

$$S = \frac{6000 (62)}{8 (8.72)}$$

Z - SECTION MODULUS

$$S = 5332 \text{ psi}$$

$$5332 < 21600 \text{ O.K.}$$

WELDMENTS OF ITEMS ② AND ①

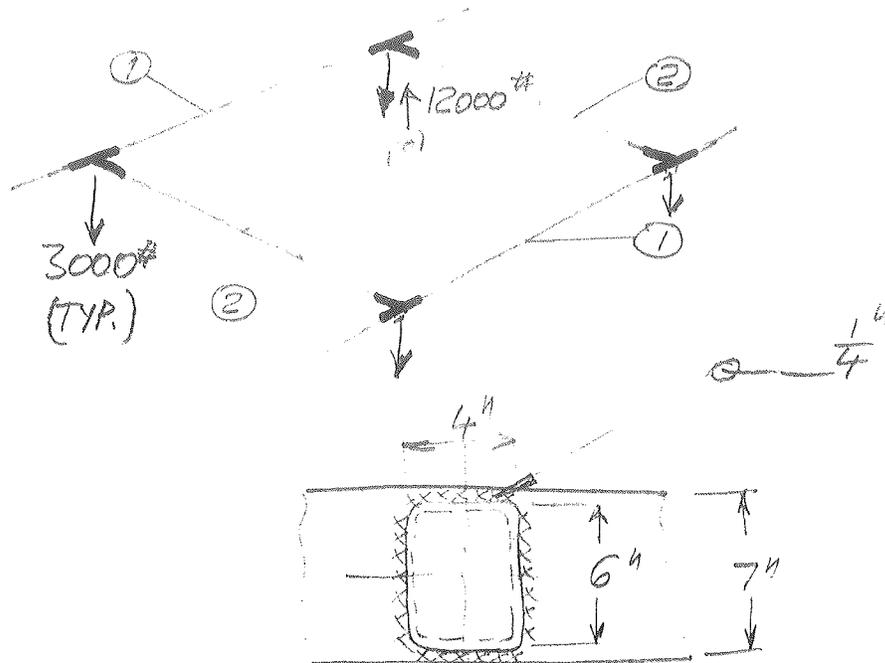


FIG. 6

ALLOWABLE STATIC LOAD PER WELDMENT AS SHOWN IN FIG. 6

NORMAL WELDS:

$$F_1 = 2500 \times 4 \times 2 = 20000 \text{ lbs}$$

PARALLEL WELDS:

$$F_2 = 2000 \times 6 \times 2 = 24000 \text{ lbs}$$

$$F_T = 20000 + 24000 = 44000 \text{ lbs}$$

O.K.

ITEMS ①:

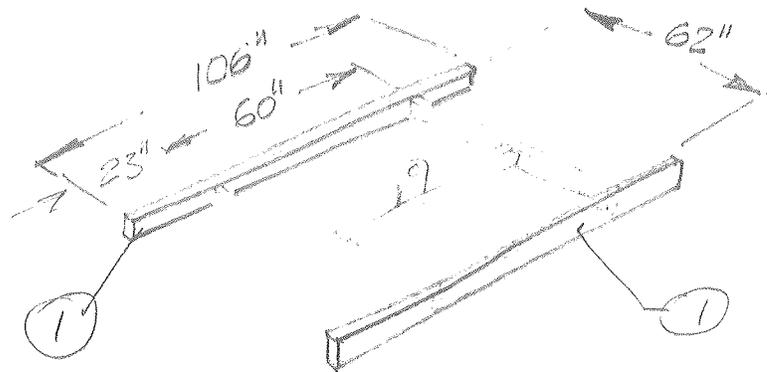


FIG. 7

<p>Case 4. — Supported at Both Ends, Two Symmetrical Loads</p>	<p>Between each support and adjacent load,  <math>s = -\frac{Wx}{Z}</math></p> <p>Between loads,  <math>s = -\frac{Wa}{Z}</math></p>	<p>Stress at each load, and at all points between,  <math>-\frac{Wa}{Z}</math></p>	<p>Between each support and adjacent load,  <math>y = \frac{Wx}{6EI} [3a(l-a) - x^2]</math></p> <p>Between loads,  <math>y = \frac{Wa}{6EI} [3v(l-v) - a^2]</math></p>	<p>Maximum deflection at center,  <math>\frac{Wa}{24EI} (3l^2 - 4a^2)</math></p> <p>Deflection at loads <math>\frac{Wa^2}{6EI} (3l - 4a)</math></p>
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ASSUMING  $W = 6000$

$$S = \frac{6000(23)}{9.23} = 14951 \text{ [psi]}$$

$$14951 < 21600$$

(TOTAL LOAD ON ONE SIDE OF THE FRAME

O.K.

DWG. NO 9210.350-ME-272106

ITEM 1.

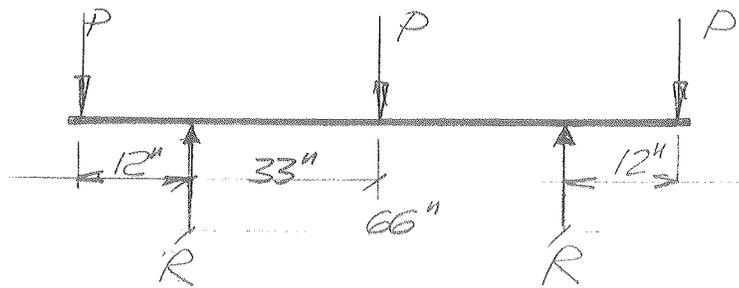
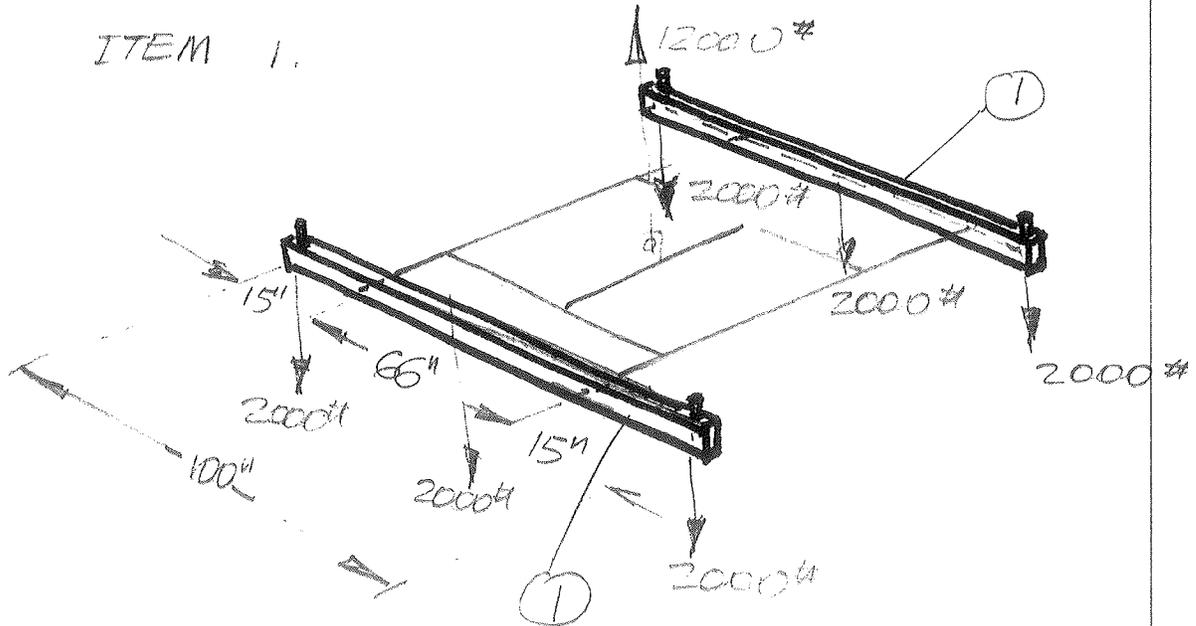
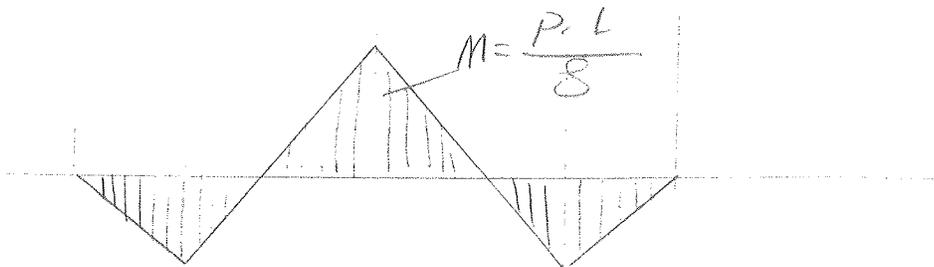


FIG. 8

$P = 2000 \text{ lbs}$

$R = 3000 \text{ lbs}$



$$M_{\text{max}} = \frac{P(L)}{8} = \frac{2000(66)}{8} = 16500 \text{ lb-in}$$

$$\sigma = \frac{M}{Z} = \frac{16500}{7.36} = 2241 \text{ [psi]}$$

CHECKING THE STRESSES IN CLAMPING DEVICE AREAS :

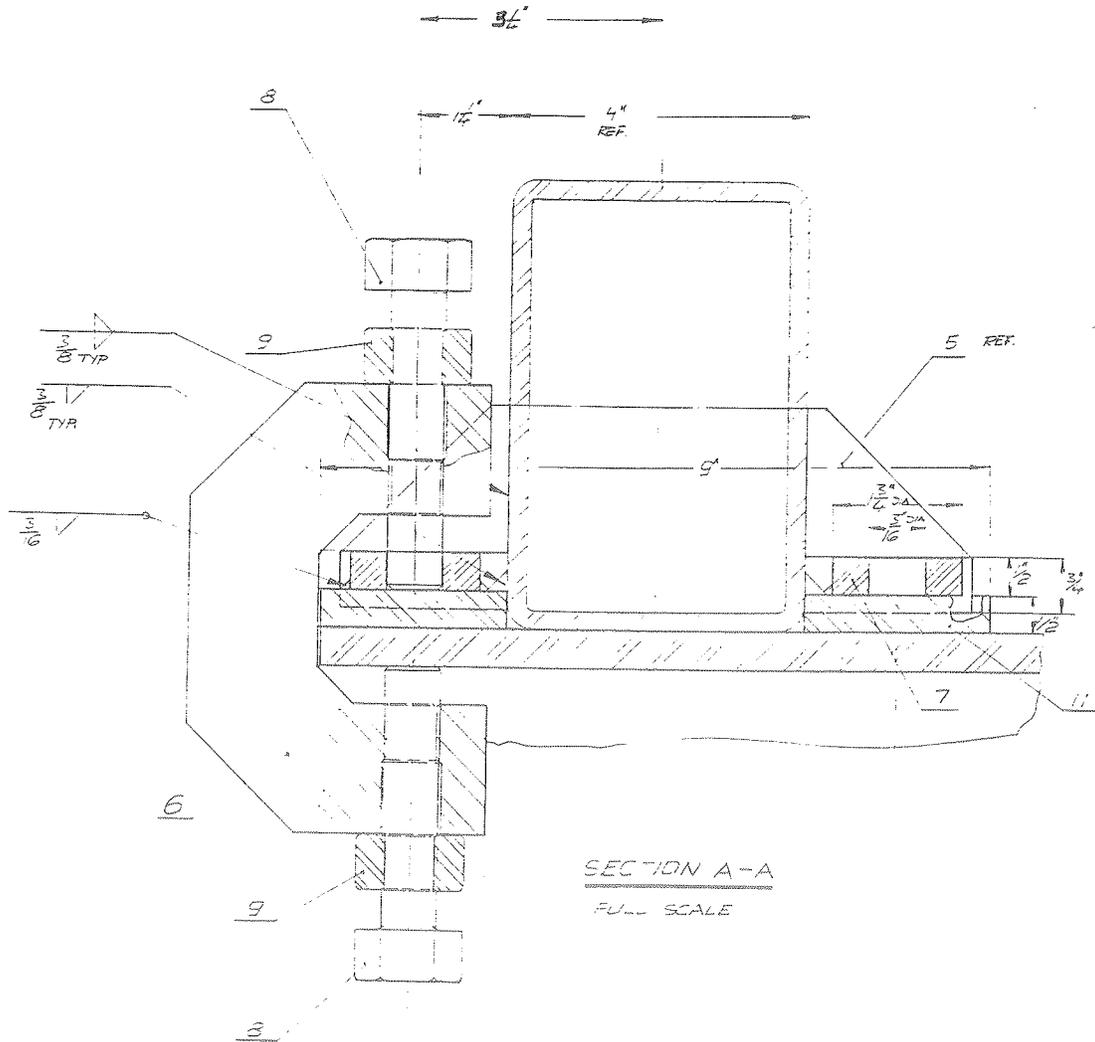


FIG. 9

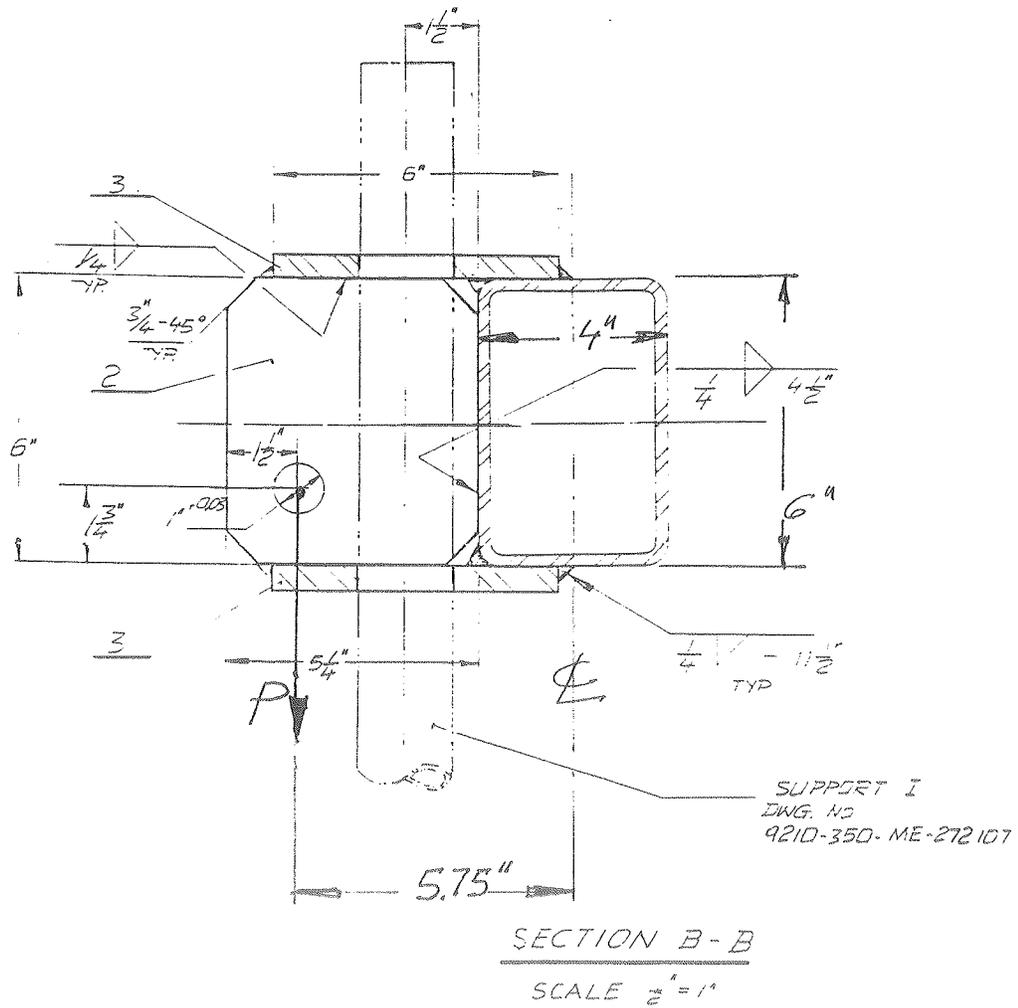


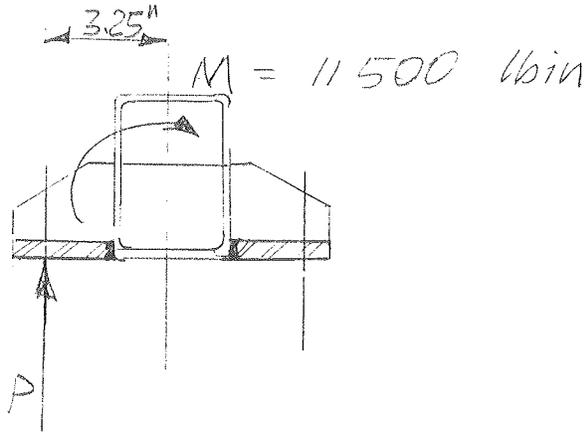
FIG. 10

DUE TO THE OFF SET OF THE FORCE "P"  
RESULTANT MOMENT WILL BE :

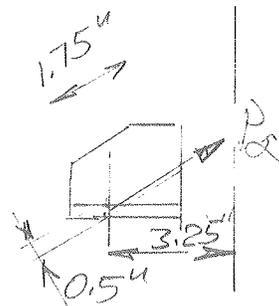
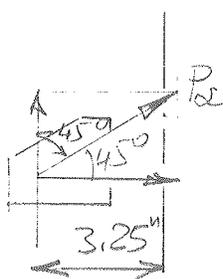
$$M = P (5.75) = 2000 (5.75)$$

$$M = 11500 \text{ [in]} \text{ ]}$$

THIS MOMENT WILL AFFECT THE CLAMPING  
STRUCTURE AND SUPPORT BRACKETS —  
AS SHOWN IN FIG. 9.



$$M = P(3.25) \quad P = \underline{3539} \text{ lbs}$$



$$\frac{P}{R} = \sin 45^\circ$$

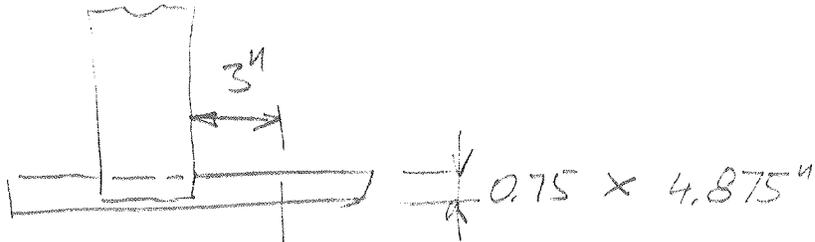
$$\frac{D}{x} = \frac{P}{\sin 45^\circ} \quad P_x = 5004 \text{ lbs}$$

$$q = \frac{D}{A} = \frac{5004}{(1.75) \times (0.5) \times (3)}$$

$$\bar{\sigma} = \underline{714} \text{ psi} \quad \text{o.k.}$$

DWG. NO 9210-350-ME-272107

ITEM No 3.



$$P = 2000 \# \quad z = \frac{bh^2}{6} = \frac{4.875 (0.75)^2}{6}$$

$$z = 0.45 \text{ in}^3$$

$$\sigma = \frac{M}{z} = \frac{2000 (3)}{0.45} = 13333 \text{ psi}$$

$$13333 < 21500 \quad \text{O.K.}$$

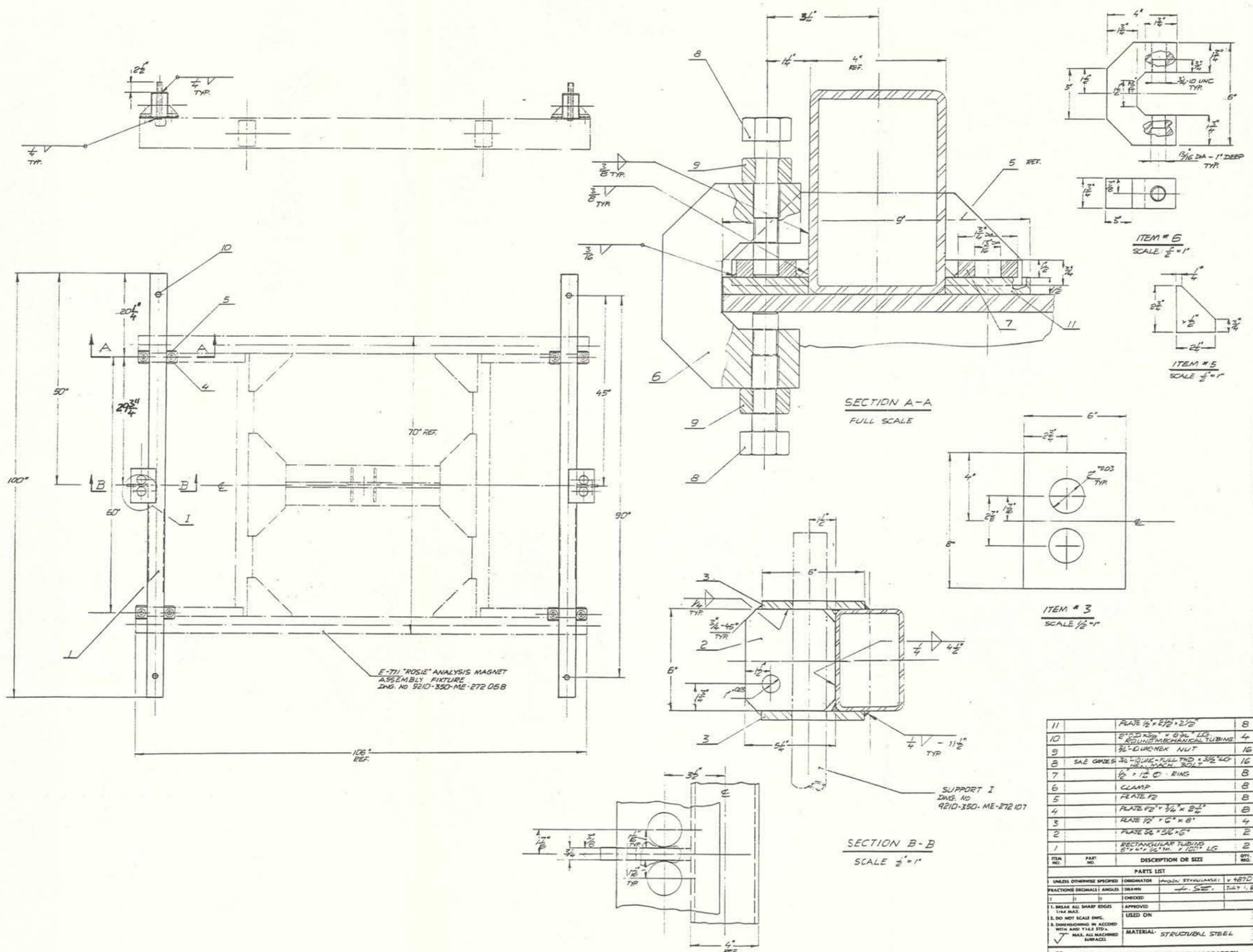
TEST LOADING ARRANGEMENT FOR FIGURE 1



I.D. No 23

TEST DATE :  
OCT. 31. 88





E-771 "ROSIE" ANALYSIS MAGNET  
ASSEMBLY FIXTURE  
DWG. NO. 9210-350-ME-272 05B

SUPPORT I  
DWS. NO.  
9210-350-ME-272 107

ITEM NO.	PART NO.	DESCRIPTION OR SIZE	QTY. REQ.
11		PLATE 1/2" x 2 1/2" x 2 1/2"	8
10		2" O.D. x 1/4" WALL x 8 3/4" LG. ROUND MECHANICAL TUBING	4
9		3/8" O.D. UNC HEX NUT	16
8		SAE GR50 3/4" O.D. - FULL THD x 3/16" LG. HEX. MACH. BOLT	16
7		1/2" x 1/2" O. RING	8
6		CLAMP	8
5		PLATE 1/2"	8
4		PLATE 1/2" x 3/4" x 2 1/2"	8
3		PLATE 1/2" x 6" x 8"	4
2		PLATE 3/4" x 3 1/2" x 6"	2
1		RECTANGULAR TUBING 6" x 4" x 1/4" TH. x 100" LG.	2

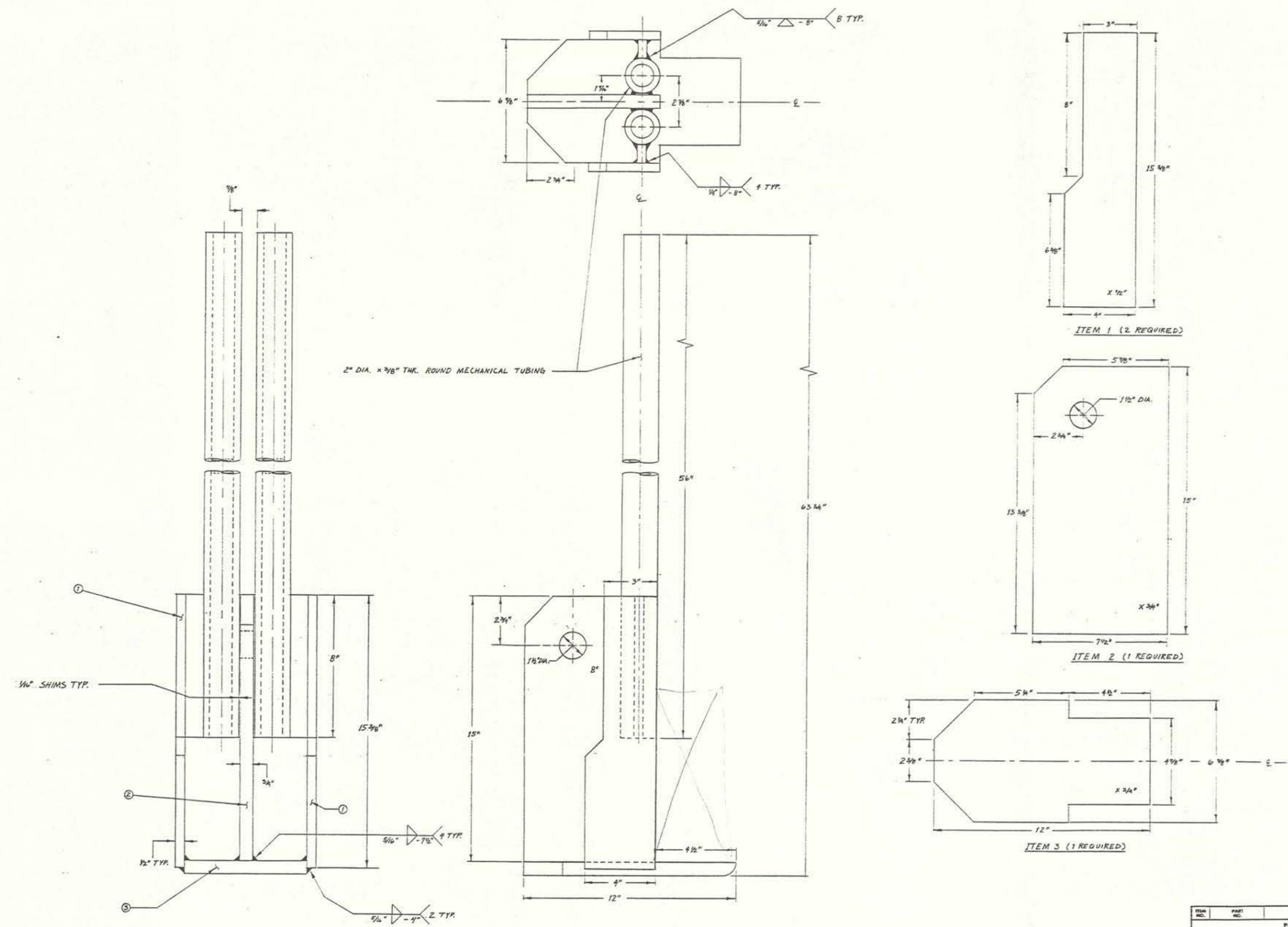
PARTS LIST  
UNLESS OTHERWISE SPECIFIED: DIMENSIONS IN INCHES  
FRACTIONS DECIMALS ANGLES DEGREES  
1. BREAK ALL SHARP EDGES 1/16" MAX.  
2. DO NOT SCALE DIMS.  
3. DIMENSIONING IN ACCORD WITH AND Y ALL STD.  
TYP. MAX. ALL MACHINED SURFACES

APPROVED: \_\_\_\_\_  
LISED ON: \_\_\_\_\_  
MATERIAL: STRUCTURAL STEEL

FERMI NATIONAL ACCELERATOR LABORATORY  
UNITED STATES DEPARTMENT OF ENERGY  
E-771 ROSIE ANALYSIS MAGNET  
ASSEMBLY FIXTURE - CTR LIFTING BEAMS  
R/D MECHANICAL

SCALE: 1/8" = 1"  
REV. AS NOTED

PLANNED: \_\_\_\_\_  
DRAWING NUMBER: 9210-350-ME-272 106  
REV. \_\_\_\_\_



ITEM NO.	PART NO.	DESCRIPTION OR SIZE	QTY.
PARTS LIST			
		FORWARDER: J.A. SETHI/JLAKSA	
		CHECKED: J. SALAS	17/16/88
		APPROVED: [Signature]	
		USED ON: [ ]	
		MATERIAL: STRUCTURAL STEEL	
FERMI NATIONAL ACCELERATOR LABORATORY UNITED STATES DEPARTMENT OF ENERGY			
E-771 ROSIE ANALYSIS MAGNET SUPPORT 1 R/D MECHANICAL			
SCALE: 1/2" = 1"	FILMED: [ ]	DRAWING NUMBER: 7210-550-ME-272107	REV: [ ]

