

DESIGN CONNECTIONS AT POINT AA, CC, HH, MM, PP, NN,  
II & DD.

From the ansys results, knowing the worst combination loadings at points mentioned above are :

ELEMENT 114-0°

$F_x = 2545.5 \text{ lb}$  (Tension)

ELEMENT 12-0°

$F_y = 1745.5 \text{ lb}$  (Shear)

ELEMENT 12-90°

$F_z = 1088.2 \text{ lb}$  (Shear)

ELEMENT 12-90°

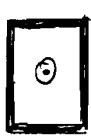
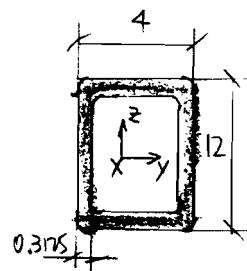
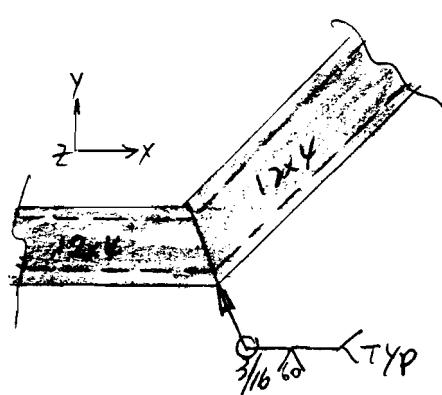
$M_x = 12575 \text{ lb-in}$  (Torsion)

ELEMENT 25-90°

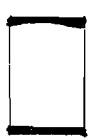
$M_y = 23657 \text{ lb-in}$  (Bending in strong axis)

ELEMENT 114-0°

$M_z = 20115 \text{ lb-in}$  (Bending in weak axis)



$F_x$



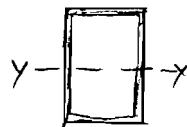
$F_y \rightarrow$



$F_z \uparrow$



torsion



$M_y$



$M_z$

According to "Design of welded structures" BLODGETT, Section 7.4-7, page 7.4-7, Table 5 - "properties of weld treated as Line", know:

Bending  $S_y = bd + \frac{d^2}{3}$  where  $b=4$   $d=12$

$$S_y = 4 \times 12 + \frac{12^2}{3} = 96 \text{ in}^2$$

Bending  $S_z = bd + \frac{d^2}{3}$  where  $b=12$   $d=4$

$$S_z = 12 \times 4 + \frac{4^2}{3} = 53.33 \text{ in}^2$$

Twisting  $J = \frac{(b+d)^3}{6}$  where  $b=4$ ,  $d=12$

$$J = \frac{(4+12)^3}{6} = 682.67 \text{ in}^3$$

## O DETERMINE THE FORCE ON WELD

Tension  $f_x = \frac{P}{A_w}$  where  $P = 2545.5 \text{ lb}$

$$A_w = 4 \times 2 + 12 \times 2 = 32 \text{ in}$$

$$f_x = \frac{2545.5}{32} = 79.54 \text{ lb/in}$$

shear  $f_y = \frac{V}{A_w}$  where  $V = 1745.5 \text{ lb}$

$$A_w = 4 + 4 = 8 \text{ in}$$

$$f_y = \frac{1745.5}{8} = 218.18 \text{ lb/in}$$

shear  $f_z = \frac{V}{A_w}$  where  $V = 1888.2 \text{ lb}$

$$A_w = 12 + 12 = 24 \text{ in}$$

$$f_z = \frac{1888.2}{24} = 78.68 \text{ lb/in}$$

Torsion  $f_{Tx} = \frac{TC_x}{J}$  where  $T = M_x = 12575 \text{ lb-in}$   $C_x = \frac{12}{2} = 6 \text{ in}$   
 $J = 682.67 \text{ in}^3$

$$f_{Tx} = \frac{(2575 \times 6)}{682.67} = 110.52 \text{ lb/in}$$

Torsion  $f_{Ty} = \frac{TC_x}{J}$  where  $T = M_x = 12575 \text{ lb-in}$   $C_x = \frac{4}{2} = 2 \text{ in}$   
 $J = 682.67 \text{ in}^3$

$$f_{Ty} = \frac{12575 \times 2}{682.67} = 36.84 \text{ lb/in}$$

Bending

$$f_x' = -\frac{M}{S_y} \quad \text{where } M = M_y = 23657 \text{ lb-in}$$

$$S_y = S_y = 96 \text{ in}^2$$

$$f_x' = \frac{23657}{96} = 246.43 \text{ lb/in}$$

Bending

$$f_x'' = -\frac{M}{S_z} \quad \text{where } M = M_z = 20115 \text{ lb-in}$$

$$f_x'' = \frac{20115}{53.33} = 377.18 \text{ lb/in}$$

## O DETERMINE WELD SIZE BY USING ALLOWABLES :

According to "Design of welded structures" Blodgett, Section 7.4, Page 7.4-7, knowing:

The resultant force is equal to the square root of the sum of the squares of these three forces.

$$f_r = \sqrt{f_1^2 + f_2^2 + f_3^2}$$

Therefore :

$$\begin{aligned}
 f_r &= \sqrt{(f_x + f_{x'} + f_{x''})^2 + (f_y + f_{Ty})^2 + (f_z + f_{Tz})^2} \\
 &= \sqrt{(79.54 + 246.43 + 377.18)^2 + (218.18 + 110.52)^2 + (78.68 + 36.84)^2} \\
 &= 784.73 \text{ lb/in} \\
 &= 0.785 \text{ kips/in}
 \end{aligned}$$

According to AISC, ASD, page 4-168, knowing, the single V-groove welds is acceptable.

According to AISC, ASD, Table J2.1 & Table J2.3, page 5-66, knowing: the effective throat thickness is  $\frac{3}{16}$ ".

According to AISC, ASD, Table J2.5, page 5-70, knowing the allowable stress on welds is the same as base metal.

According to ANSI/ASME, B30-20-1993, Below the Hook Lifting Devices, knowing, the allowable stress is equal to  $\frac{1}{3} F_y$ .

where  $F_y = 36 \text{ ksi}$  for A36.

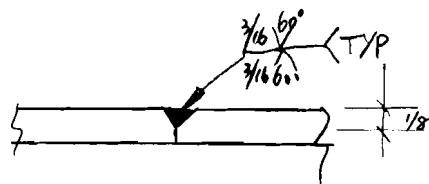
$$\frac{1}{3} F_y = \frac{1}{3} \times 36 = 12 \text{ ksi}$$

Therefore, the allowable loads in kips/in is equal to

$$12 \times 0.707 \times \frac{3}{16} = 1.59 \text{ kips/in.}$$

use the continuous weld.

$$SF = \frac{1.59}{0.785} = 2.0,$$



WELDS: E70XX

In conclusion, design as sketched is adequate to carry the load.

## ~~STRONGBACK~~ CONNECTION WEIGHT

$$\text{Total weight} = W_{\text{plate+angle}} + W_{\text{Bolts+nuts}}$$

where  $W_{\text{plate+angle}} = 1543 \text{ lb}$

$$W_{\text{Bolts+nuts}} = 108.672 \text{ lb}$$

$$\begin{aligned}\text{Total weight} &= 1543 + 108.672 \\ &= 1652 \text{ lb}\end{aligned}$$

~~PLATES USED (COMBINED PLATES & ANGLES)~~

- 1).  $0.75'' \times 1.5'' \times 4'' \rightarrow 1\text{b} (\text{each})$
- 2).  $0.375 \times 4'' \times 7'' \rightarrow 12 (\text{each})$
- 3).  $L 4 \times 4 \times 8\frac{1}{2}'' \times 3\frac{1}{8}'' \rightarrow 24 (\text{each})$
- 4).  $L 4 \times 7 \times 8\frac{1}{2}'' \times 3\frac{1}{8}'' \rightarrow 18 (\text{each})$
- 5).  $0.5'' \times 5'' \times 10'' \rightarrow 18 (\text{each})$
- 6).  $0.875'' \times 11\frac{7}{8}'' \times 11\frac{7}{8}'' \rightarrow 2 (\text{each})$
- 7).  $0.875'' \times 11\frac{7}{8}'' \times 8\frac{1}{2}'' \rightarrow 7 (\text{each})$
- 8).  $0.625 \times 11\frac{7}{16}'' \times 6\frac{1}{2}'' \rightarrow 4 (\text{each})$
- 9).  $1.25 \times 11\frac{7}{16}'' \times 30'' \rightarrow 2 (\text{each})$
- 10).  $0.5 \times 10.5 \times 12 \rightarrow 2 (\text{each})$

— Weight

- 1).  $0.475 \times 16 = 6.8 \text{ lb}$
- 2).  $1.488 \times 12 = 17.86 \text{ lb}$
- 3).  ~~$9.8 \frac{\text{lb}}{\text{ft}} \times 17 \text{ ft} = 166.6 \text{ lb}$~~
- 4).  $13.6 \frac{\text{lb}}{\text{ft}} \times 12.75 \text{ ft} = 173.4 \text{ lb}$
- 5).  $7.08 \times 18 = 127.60 \text{ lb}$
- 6).  $34.985 \times 2 = 69.97 \text{ lb}$
- 7).  $25.04 \times 7 = 175.31 \text{ lb}$
- 8).  $131.75 \times 4 = 527 \text{ lb}$
- 9).  $121.62 \times 2 = 243.25 \text{ lb}$
- 10).  $17.5 \times 2 = 35 \text{ lb}$

total : 1543 lb

— A325 Bolt & Nut list.

1).  $\frac{3}{4}'' - 2\frac{1}{4}$  long — 68 (ea)

2).  $\frac{3}{4}'' - 5$  long — 4 (ea)

3).  $\frac{3}{4}'' - 5\frac{1}{2}$  long — 16 (ea)

4).  $\frac{3}{4}'' - 7$  long — 36 (ea)

— A325 Bolt & Nut weight

1).  $\frac{3}{4}'' \times 2\frac{1}{4} \rightarrow 0.648 \times 68 = 44 \text{ lb}$

2).  $\frac{3}{4}'' \times 5 \rightarrow 0.958 \times 4 = 3.832 \text{ lb}$

3).  $\frac{3}{4}'' \times 5\frac{1}{2} \rightarrow 1.08 \times 16 = 17.28 \text{ lb}$

4).  $\frac{3}{4}'' \times 7 \rightarrow 1.21 \times 36 = 43.56 \text{ lb}$

— Total Bolt & Nut weight

108.672 lb