

**BELOW-THE-HOOK LIFTING DEVICE**  
**Engineering Note Cover Page**

Lifting Device Numbers:

 FNAL Site No/ \_\_\_\_\_ Div. Specific No. 141 Asset No. \_\_\_\_\_

If applicable

If applicable

If applicable

 ASME B30.20 Group: (check one)
 

<input checked="" type="checkbox"/> Group I	Structural and Mechanical Lifting Devices
<input type="checkbox"/> Group II	Vacuum Lifting Devices
<input type="checkbox"/> Group III	Magnets, Close Proximity Operated
<input type="checkbox"/> Group IV	Magnets, Remote Operated

Device Name or Description

NUMI WATER SKID INST FIXTURE
 Device was  Purchased from a Commercial Lifting  
 Device Manufacturer. Mfg Name \_\_\_\_\_

 (check all applicable)  Designed and Built at Fermilab

 Designed by Fermilab and Built by a  
 Vendor. Assy drawing number 8875.117-MD-427375
 Provided by a User or other Laboratory Other: Describe \_\_\_\_\_Engineering Note Prepared by D. PUSHKADate 8 Jan 2004Engineering Note Reviewed by A. LeeDate 1-12-2004

Lifting Device Data:

Capacity 4000 #Fixture Weight 300 #
 Service:  normal  heavy  severe (refer to B30.20 for definitions)

Duty Cycle \_\_\_\_\_ 8, 16 or 24 hour rating (applicable to groups III, and IV)

Inspections Frequency BEFORE USERated Load Test by FNAL (if applicable) Date 1/15/04 Load 5000 # Check if Load Test was by Vendor and attach the certificateSatisfactory Load Test Witnessed by: JOHN VOIRIN

Signature (of Load Test Witness)

John Voirin 01/19/04
 Notes or Special Information: USED FOR TARGET, HORN 1, HORN 2, DECAY U.S.,  
DECAY D.S., ABSORBER RAW, ABSORBER INTERMEDIATE, & MINOS  
LCW SKIDS. - TWO LIFTING FIXTURES HAVE BEEN BUILT.

NUMI WATER SKID INSTALLATION FIXTURE WAS SIZED FOR 2500# CAPACITY. ACTUAL WEIGHT OF SKIDS IS 3000 TO 4000#. AS MEASURED IN NEW MUON.

THESE CALCULATIONS ARE TO SHOW THE EXISTING DESIGN IS GOOD FOR THE HIGHER WEIGHTS.

### BOLTS:

1/2-13 BOLTS ATTACH FIXTURE TO SKID. ASSUME LOAD IS EQUALLY SHARED BY EIGHT BOLTS.

$$P = \frac{4000 \#}{8 \text{ BOLTS}} = 500 \#. \quad \text{SINGLE SHEAR}$$

$$T = \frac{P}{A} = \frac{500 \#}{\frac{\pi}{4} (0.5)^2} = 2546 \text{ PSI}$$

ALLOWABLE STRESS FOR CLASS 2 (CHEAP) BOLTS

$$\text{IS } \frac{F_y}{3} = \frac{45,000 \text{ PSI}}{3} = 15,000 \text{ PSI}$$

ACTUAL STRESS < ALLOWABLE STRESS  $\therefore$  OK

VERTICAL CHANNELS:

C4 x 7.25 CHANNEL IS USED TO CARRY LOAD IN TENSION.

$$A = 2.13 \text{ in}^2$$

$$\sigma_{\text{ALLOW}} = \frac{F_y}{3} = \frac{36,000 \text{ psi}}{3} = 12,000 \text{ psi}$$

ASSUME 100% OF LOAD IS ON 1 CHANNEL:

$$\sigma_{\text{ACTUAL}} = \frac{4000 \#}{2.13 \text{ in}^2} = 1877 \text{ psi}$$

$$\sigma_{\text{ACTUAL}} < \sigma_{\text{ALLOWABLE}} \therefore \text{OK}$$

ASSUME 100% OF LOAD IS ON 2 BOLTS @ TOP OF CHANNEL. 2 @ 1/2-13 SCREWS

$$A = (2) \frac{\pi}{4} (.5^2) = 0.39 \text{ in}^2$$

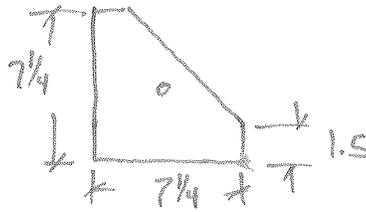
$$\sigma = \frac{P}{A} = \frac{4000 \#}{0.39} = 10,185 \text{ psi}$$

$$\sigma_{\text{ALLOW}} = \frac{F_y}{3} = \frac{45,000}{3} = 15,000 \text{ psi}$$

$$\sigma_{\text{ACTUAL}} < \sigma_{\text{ALLOWABLE}} \therefore \text{OK}$$

LIFTING PLATE:

3/4" THICK A36



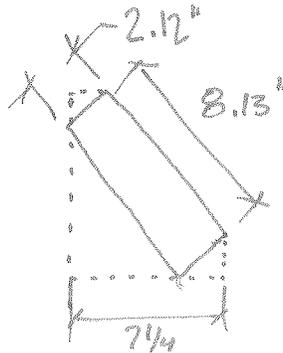
$$\sigma_{\text{ALLOWABLE}} = \frac{F_y}{3} = \frac{36,000 \text{ psi}}{3} = 12,000 \text{ psi}$$

CONSIDER AS A BEAM:

$$l = 8.13''$$

$$b = 3/4''$$

$$d = 2.12''$$



$$I = \frac{1}{12} d b^3 = \frac{1}{12} \cdot 2.12 \cdot (3/4)^3$$

$$= 0.0746 \text{ in}^4$$

$$M = \frac{Pl}{4}; \quad \sigma = \frac{My}{I}; \quad y = 3/8''$$

$$\sigma = \frac{Pl}{4} \cdot \frac{3/8 \text{ in}}{0.0746 \text{ in}^4} = P \times 8.13 \text{ in} \cdot \frac{1}{4} \cdot \frac{3}{8 \text{ in}} \cdot \frac{1}{0.0746 \text{ in}^4}$$

$$\sigma = P \cdot 10.217 \text{ (psi)}$$

IF LOAD IS EQUALLY DISTRIBUTED,  $P = \frac{4000 \#}{4 \text{ LEGS}} = 1000 \#$

$$\sigma_{\text{ACTUAL}} = (1000 \#)(10.217 \text{ in}^2) = 10,216 \text{ psi}$$

$$\sigma_{\text{ACTUAL}} < \sigma_{\text{ALLOWABLE}} \therefore \text{OK}$$

7 JAN 2004

D. PUSHKA

LIFTING SLINGS:

SKID WILL BE LIFTED WITH 4 SLINGS.

NYLON SLINGS DO STRETCH A BIT, THEREFORE

THE LOAD WILL BE DIVIDED BY THE FOUR

SLINGS AND NOT JUST 2 AS POSTULATED

BY ED LAVALLIE'S NOTE.

AN INSPECTION OF THE SKIDS SHOW THAT

THE C.G. IS NEAR THE GEOMETRIC CENTER

OF THE SKID, SO IT IS REASONABLE TO ASSUME

THE LOAD IS EQUALLY SHARED BY THE FOUR

LIFTING POINTS.

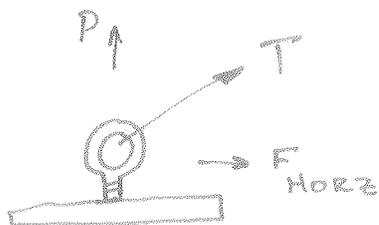
EYEBOLTS :

LIFTING PLATES ARE LIFTED VIA  $\frac{1}{2}$ -13 EYE-BOLTS.

ALLOWABLE  
CATALOG CAPACITY OF FORGED EYE BOLTS,  $\frac{1}{2}$ -13  
IS 2500# IN THE VERTICAL

EVEN IF THE VERTICAL LOAD WAS TAKEN BY ONLY  
2 EYE BOLTS,  $P = \frac{4000}{2} = 2000\#$  THE LOAD  
WOULD BE LESS THAN THE ALLOWABLE CAPACITY  
 $\therefore$  OK

ASSUME  $45^\circ$  ON SLINGS

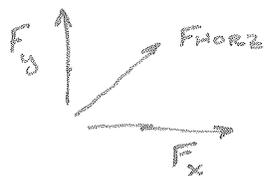


$$T = \frac{P}{\cos 45} = \frac{P}{.707}$$

$$P = \frac{4000 \# / \text{SKID}}{4 \text{ BOLTS} / \text{SKID}} = 1000 \# \text{ PER EYEBOLT}$$

$$T = \frac{P}{\sin 45} = \frac{1000 \#}{.707} = 1,414 \#$$

$$F_{\text{HORIZ}} = 1000 \#$$



$$F_x = F_y = 707 \#$$

$\therefore$  COMPRESSION IN THE  $5 \times 6.7$  CHANNELS IS 707#

SIDE CHANNELS:

$$\text{COMPRESSION} = 707 \#$$

$$C5 \times 6.7 \rightarrow A = 1.97 \text{ in}^2, \quad r_{x-x} = 1.95, \quad r_{y-y} = 0.493$$

$$\therefore \text{COMPRESSIVE STRESS} = \frac{P}{A} = \frac{707 \#}{1.97 \text{ in}^2} = 358 \text{ psi}$$

## ALLOWABLE STRESS:

$$\text{length} = 60 \text{ inches}$$

$$K = 2.0$$

$$C_c = \sqrt{\frac{2\pi^2 E'}{F_y}} = \sqrt{\frac{2\pi^2 \times 29 \times 10^6}{36,000}} = 126.09 \text{ in}$$

$$\frac{KL}{r} = \frac{(2.0)(60)}{1.95} = 61.5$$

$$= \frac{(2.0)(60)}{.493} = 243.4 \leftarrow \text{DRIVES}$$

$$KL/r > C_c \therefore F_a = \frac{12 \pi^2 E}{23 (KL/r)^2}$$

$$= \frac{12 \cdot \pi^2 \cdot 29,000,000}{23 \cdot (2.0 \cdot 60 / .493)^2}$$

$$= 2520 \text{ psi}$$

ACTUAL STRESS (358 psi) < ALLOWABLE STRESS (2520 psi)

$\therefore$  OK

8 Jan 2004

D. PUSHKA

WELDS BTW LIFTING PLATE &amp; CHANNELS

$$\text{LOAD} = 1000 \#$$

WELD MATERIAL HAS A 70,000 PSI YIELD.  $\therefore$  ALLOWABLE

$$\text{STRESS} = \frac{70,000}{3} = 23,000 \text{ PSI} = F_v$$

 $\frac{1}{4}$ " FILLER WELDS ARE USED.

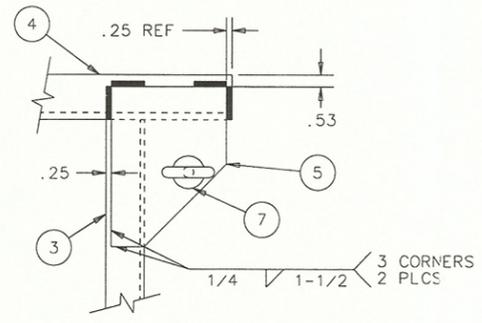
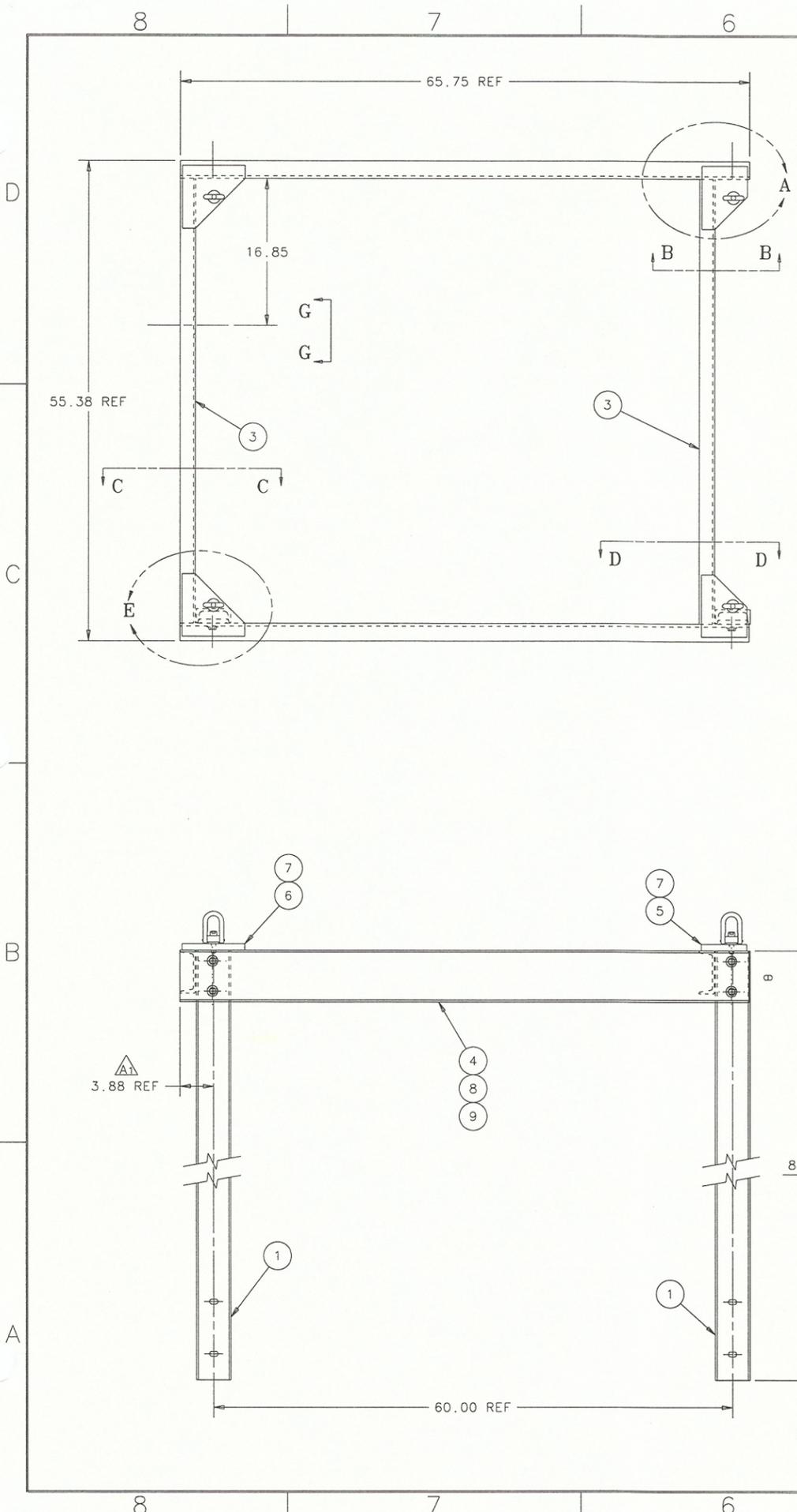
$$P = A \cdot F_v = 0.707 \cdot \frac{1}{4} \cdot l \cdot F_v$$

$$1000 \# = 0.707 \cdot \frac{1}{4} \cdot l \cdot 23,000$$

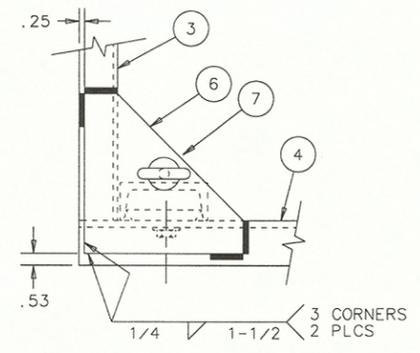
$$\therefore l_{\text{min}} = \frac{1000}{.707 \cdot \frac{1}{4} \cdot 23,000} = 0.246 \text{ IN}$$

ACTUAL WELD LENGTH USED IS  $6 \cdot \frac{1}{2} \text{ IN} = 9 \text{ IN}$ 

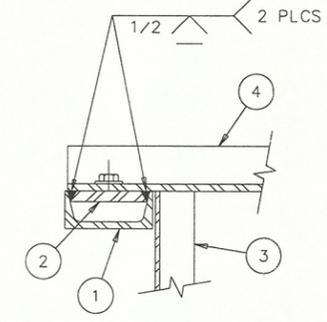
$$9 \text{ IN} > 0.246 \text{ IN} \quad \therefore \text{OK}$$



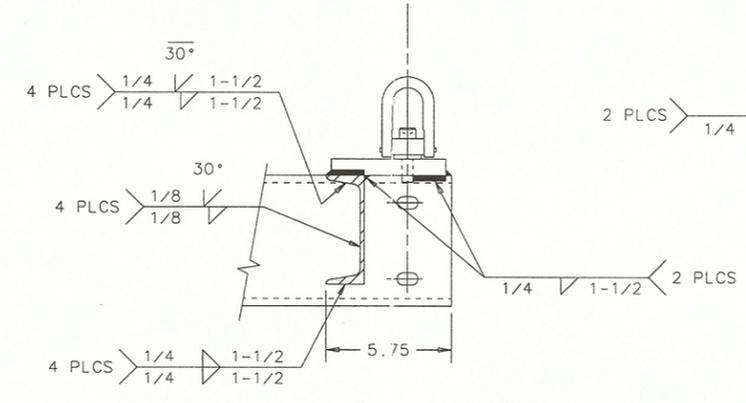
**DETAIL A**  
SCALE: 1/4



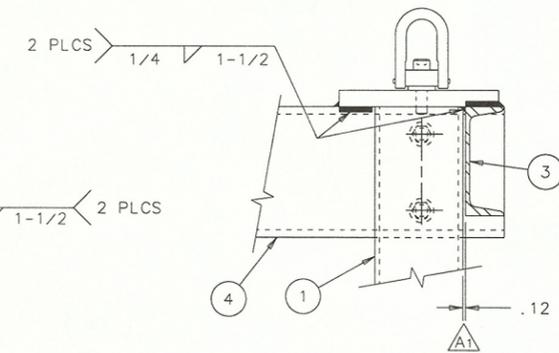
**DETAIL E**  
SCALE: 1/4



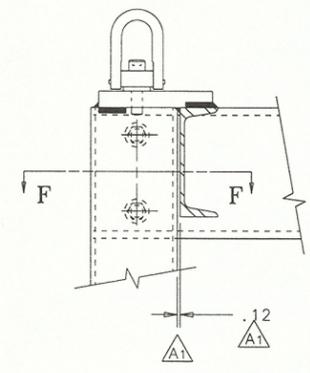
**SECTION F-F**  
SCALE: 1/4



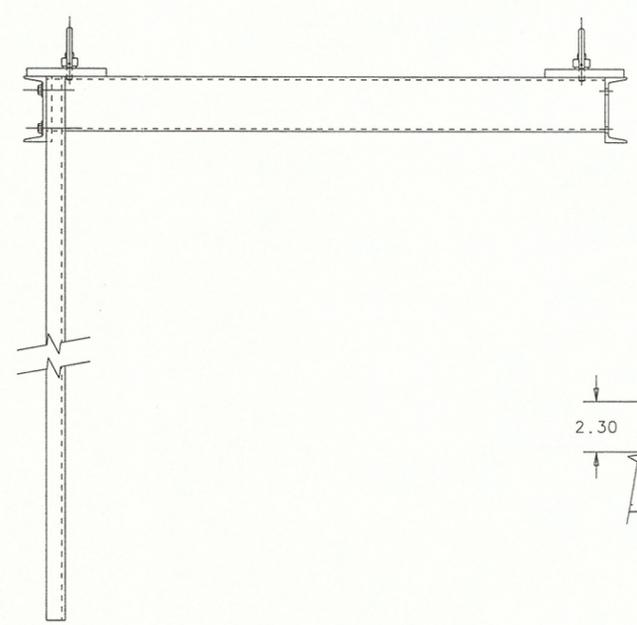
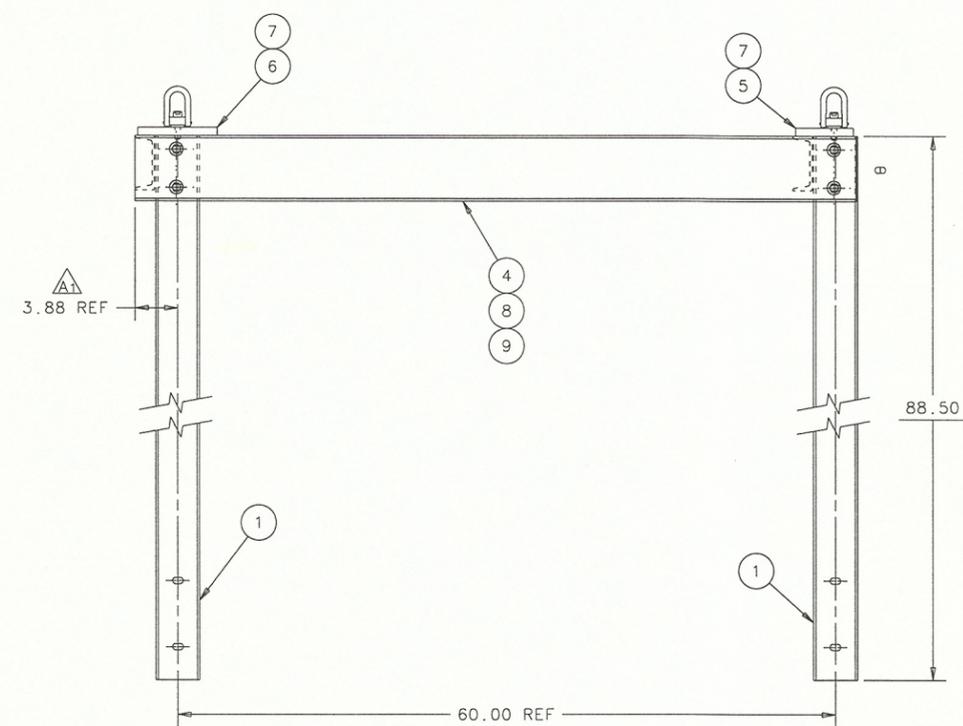
**SECTION B-B**  
SCALE: 1/4



**SECTION C-C**  
SCALE: 1/4



**SECTION D-D**  
SCALE: 1/4



**VIEW G-G**  
SCALE: 1/4

REV	DESCRIPTION	DRAWN	DATE
		APPROVED	DATE
A	1. ADDED .12 GAP 2. WAS 3.75	F. BROWNING	4-MAR-2003

ITEM	PART NO.	DESCRIPTION OR SIZE	QTY.
9	COML	WASHER, FLAT 1/2 BOLT TYPE 18-8 SS	4
8	COML	BOLT, 1/2-13UNC X 1 LG HX. HD. TYPE 18-8 SS	4
7	COML	SWIVEL HOIST RING, 1/2"-13 UNC, 2,500# RATED LOAD, REID TOOL #SHR-23301	4
6	MB-427374	LIFTING PLATE "B"	2
5	MB-427373	LIFTING PLATE "A"	2
4	MC-427371	REAR CHANNEL - "A"	2
3	COML	CHANNEL, C5 x 6.7 x 51.32 LG. ASTM A36	2
2	MB-427372	MOUNTING PLATE	2
1	MC-427370	VERTICAL CHANNEL - "A"	2

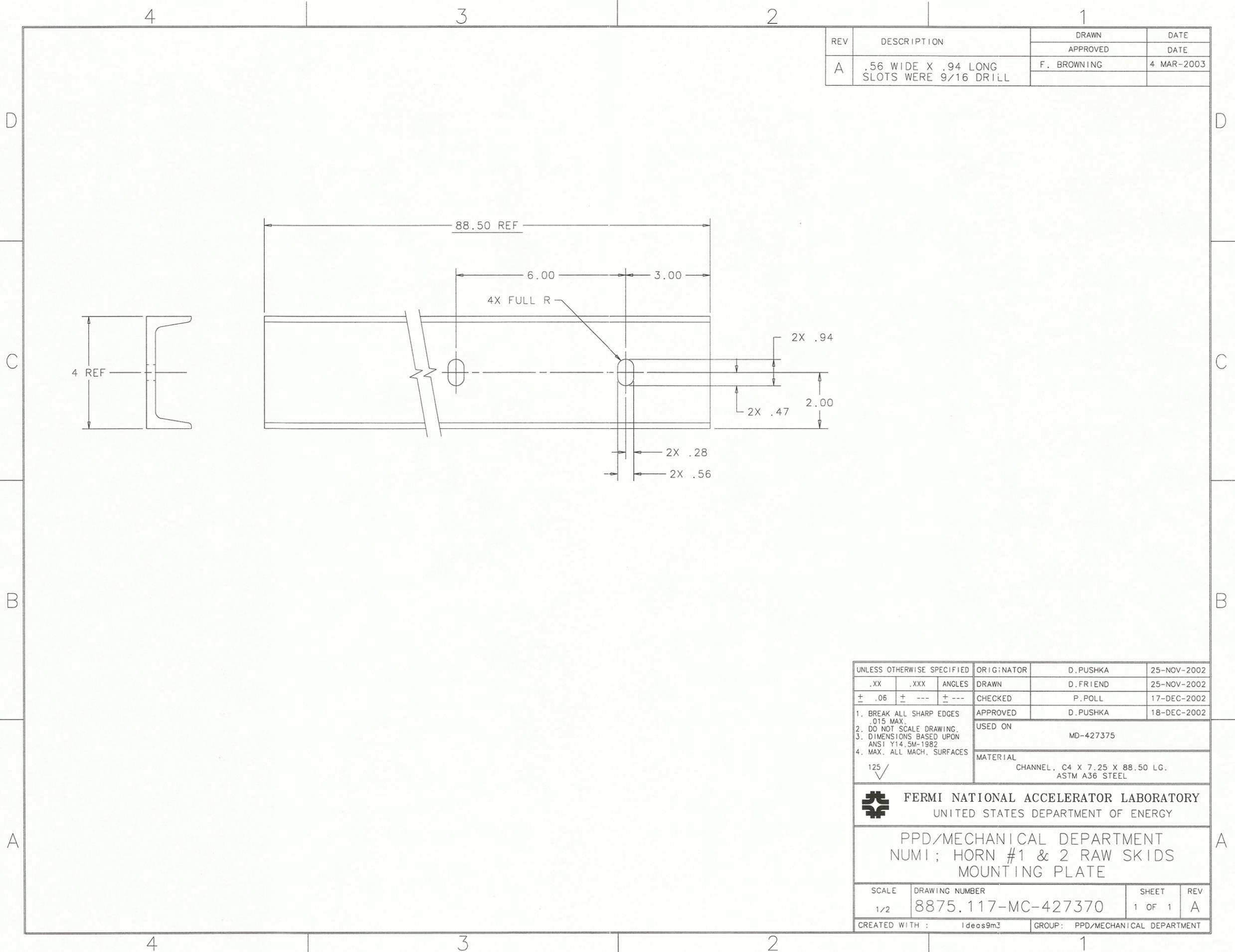
PARTS LIST			
UNLESS OTHERWISE SPECIFIED	ORIGINATOR	D.PUSHKA	02-DEC-2002
.XX .XXX ANGLES	DRAWN	D.FRIEND	02-DEC-2002
± .06 ± --- ± ---	CHECKED	P.POLL	18-DEC-2002
1. BREAK ALL SHARP EDGES .015 MAX. 2. DO NOT SCALE DRAWING. 3. DIMENSIONS BASED UPON ANSI Y14.5M-1982 4. MAX. ALL MACH. SURFACES	APPROVED	D.PUSHKA	18-DEC-2002
USED ON ME-406249			
MATERIAL SEE PARTS LIST ABOVE			

**Fermi National Accelerator Laboratory**  
UNITED STATES DEPARTMENT OF ENERGY

PPD/MECHANICAL DEPARTMENT  
NUM1; HORN #1 & 2 RAW SKID  
LIFTING FIXTURE ASSEMBLY #2

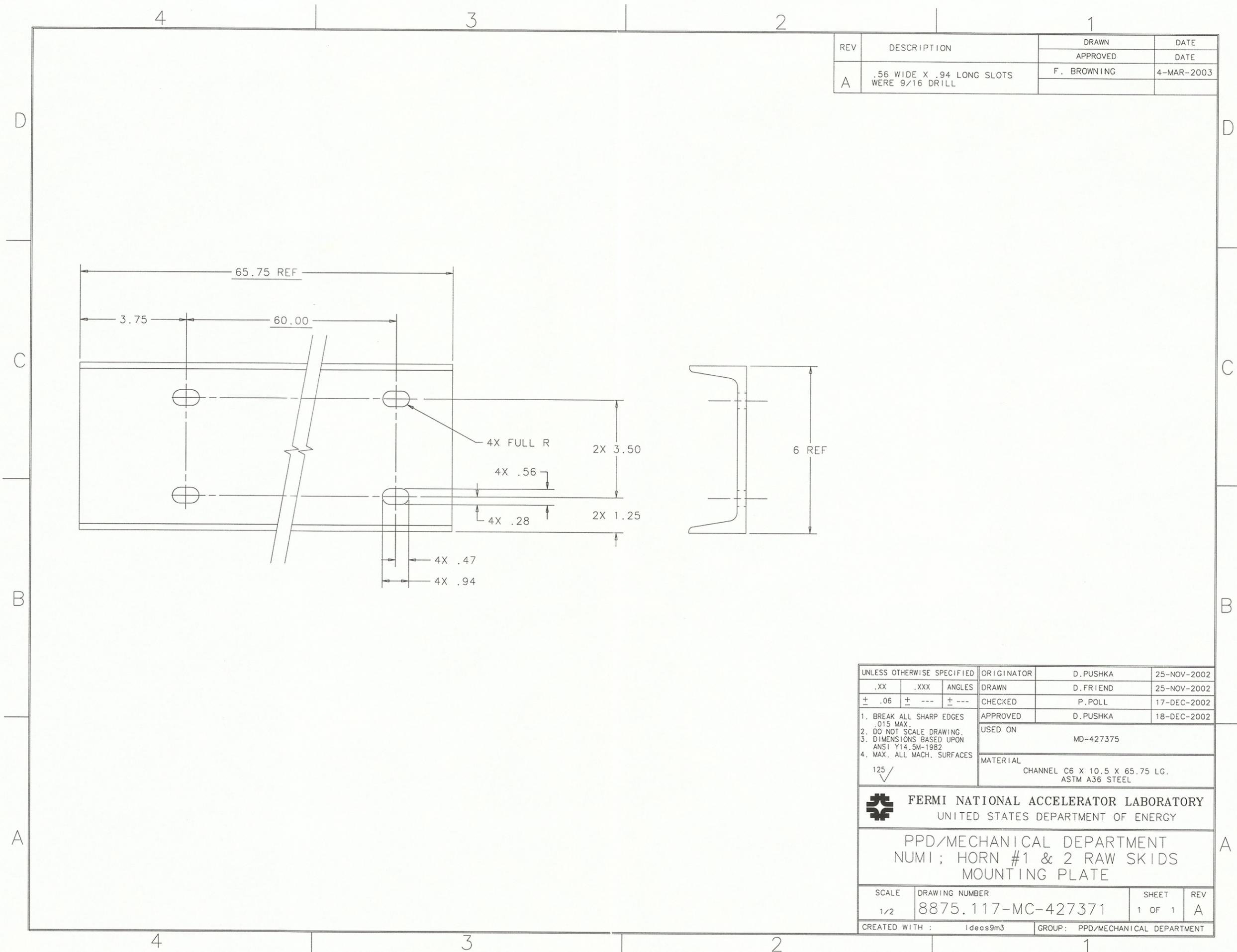
SCALE	DRAWING NUMBER	SHEET	REV
1/8	8875.117-MD-427375	1 OF 1	A

CREATED WITH: Idec9m3 GROUP: PPD/MECHANICAL DEPARTMENT



REV	DESCRIPTION	DRAWN	DATE
		APPROVED	DATE
A	.56 WIDE X .94 LONG SLOTS WERE 9/16 DRILL	F. BROWNING	4 MAR-2003

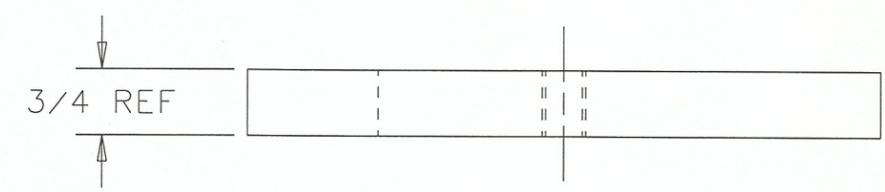
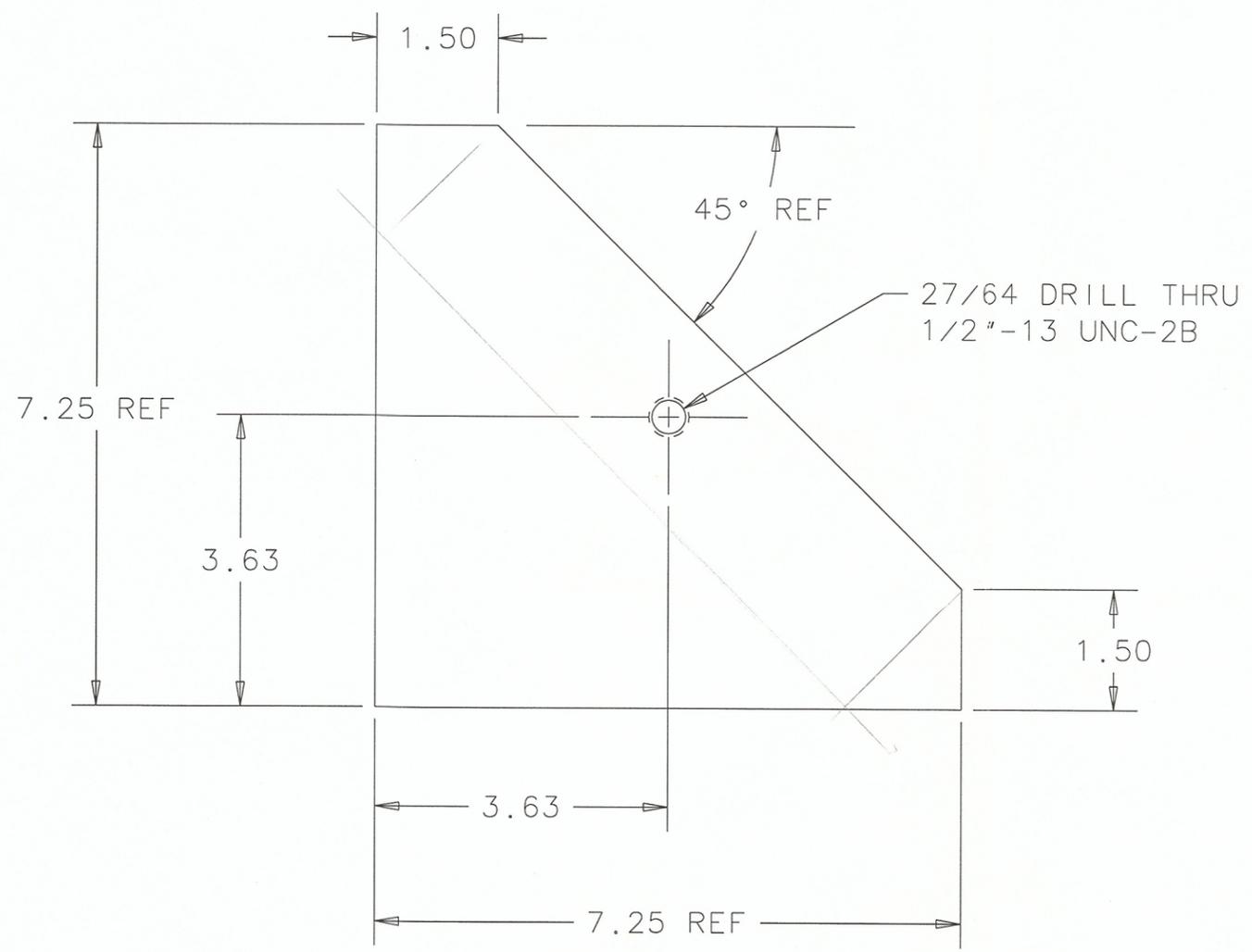
UNLESS OTHERWISE SPECIFIED			ORIGINATOR	D.PUSHKA	25-NOV-2002
.XX	.XXX	ANGLES	DRAWN	D.FRIEND	25-NOV-2002
± .06	± ---	± ---	CHECKED	P.POLL	17-DEC-2002
1. BREAK ALL SHARP EDGES .015 MAX.			APPROVED	D.PUSHKA	18-DEC-2002
2. DO NOT SCALE DRAWING.			USED ON		
3. DIMENSIONS BASED UPON ANSI Y14.5M-1982			MD-427375		
4. MAX. ALL MACH. SURFACES			MATERIAL		
125 ✓			CHANNEL, C4 X 7.25 X 88.50 LG. ASTM A36 STEEL		
 <b>FERMI NATIONAL ACCELERATOR LABORATORY</b> UNITED STATES DEPARTMENT OF ENERGY					
PPD/MECHANICAL DEPARTMENT NUMI; HORN #1 & 2 RAW SKIDS MOUNTING PLATE					
SCALE	DRAWING NUMBER			SHEET	REV
1/2	8875.117-MC-427370			1 OF 1	A
CREATED WITH : Ideas9m3			GROUP: PPD/MECHANICAL DEPARTMENT		



REV	DESCRIPTION	DRAWN	DATE
		APPROVED	DATE
A	.56 WIDE X .94 LONG SLOTS WERE 9/16 DRILL	F. BROWNING	4-MAR-2003

UNLESS OTHERWISE SPECIFIED			ORIGINATOR	D.PUSHKA	25-NOV-2002
.XX	.XXX	ANGLES	DRAWN	D.FRIEND	25-NOV-2002
± .06	± ---	± ---	CHECKED	P.POLL	17-DEC-2002
1. BREAK ALL SHARP EDGES .015 MAX.			APPROVED	D.PUSHKA	18-DEC-2002
2. DO NOT SCALE DRAWING.			USED ON		
3. DIMENSIONS BASED UPON ANSI Y14.5M-1982			MD-427375		
4. MAX. ALL MACH. SURFACES 125 ✓			MATERIAL		
			CHANNEL C6 X 10.5 X 65.75 LG. ASTM A36 STEEL		
 <b>FERMI NATIONAL ACCELERATOR LABORATORY</b> UNITED STATES DEPARTMENT OF ENERGY					
PPD/MECHANICAL DEPARTMENT NUMI; HORN #1 & 2 RAW SKIDS MOUNTING PLATE					
SCALE	DRAWING NUMBER		SHEET	REV	
1/2	8875.117-MC-427371		1 OF 1	A	
CREATED WITH : Ideas9m3			GROUP: PPD/MECHANICAL DEPARTMENT		

REV	DESCRIPTION	DRAWN	DATE
		APPROVED	DATE



UNLESS OTHERWISE SPECIFIED			ORIGINATOR	D. PUSHKA	18-NOV-2002
.XX	.XXX	ANGLES	DRAWN	D. FRIEND	18-NOV-2002
± .03	± ---	± 1°	CHECKED	P. POLL	17-DEC-2002
1. BREAK ALL SHARP EDGES .015 MAX. 2. DO NOT SCALE DRAWING. 3. DIMENSIONS BASED UPON ANSI Y14.5M-1982 4. ALL DIMENSIONS ARE INCHES 5. MAX. ALL MACH. SURFACES			APPROVED	D. PUSHKA	18-DEC-2002
125/			USED ON	MD-427375	
			MATERIAL	PLATE 3/4" X 7.25 X 7.25 ASTM A36	

 FERMIONATIONAL ACCELERATOR LABORATORY  
UNITED STATES DEPARTMENT OF ENERGY

PPD/MECHANICAL DEPARTMENT  
NUMI; HORN #1 & 2 RAW SKID  
LIFTING PLATE "B"

SCALE	DRAWING NUMBER	SHEET	REV
1/2	8875.117-MB-427374	1 OF 1	
CREATED WITH : Ideas9m3		GROUP: PPD/MECHANICAL DEPARTMENT	

**BELOW-THE-HOOK LIFTING DEVICE**  
**Engineering Note Cover Page**

Lifting Device Numbers:

FNAL Site No/ \_\_\_\_\_ Div. Specific No. 141 Asset No. \_\_\_\_\_  
 If applicable If applicable If applicable

ASME B30.20 Group:  Group I Structural and Mechanical Lifting Devices  
 (check one)  Group II Vacuum Lifting Devices  
 Group III Magnets, Close Proximity Operated  
 Group IV Magnets, Remote Operated

Device Name or Description

SWAN HOOD #1 1/2" RAW SPIND LIFTING FIXTURE

Device was  Purchased from a Commercial Lifting  
 Device Manufacturer. Mfg Name \_\_\_\_\_  
 (check all  Designed and Built at Fermilab  
 applicable)  Designed by Fermilab and Built by a  
 Vendor. Assy drawing number 8879.117.110-406223  
 Provided by a User or other Laboratory  
 Other: Describe \_\_\_\_\_

Engineering Note Prepared by ED LAVALLE Date 11/2/02

Engineering Note Reviewed by Ang Lee Date 5/16/03

Lifting Device Data:

Capacity 2500<sup>lb</sup>Fixture Weight 301<sup>lb</sup>

Service:  normal  heavy  severe (refer to B30.20 for definitions)

Duty Cycle \_\_\_\_\_ 8, 16 or 24 hour rating (applicable to groups III, and IV)

Inspections Frequency \_\_\_\_\_

Rated Load Test by FNAL (if applicable) Date \_\_\_\_\_ Load \_\_\_\_\_

Check if Load Test was by Vendor and attach the certificate

Satisfactory Load Test Witnessed by: \_\_\_\_\_

Signature (of Load Test Witness) \_\_\_\_\_

Notes or Special Information:

NUMI HORN #1 & HORN #2 RAW SKIDS LIFTING FIXTURE  
SUPPORT FRAMES IDENTICAL FOR BOTH

SKID WELDMENT WEIGHT AS PER  
DRAWING #8875.117-ME-406219

01. Channel MC12 x 10.6# x 84" long = 74.2# x 2 pieces	= 148.40#
02. Channel MC12 x 10.6# x 45" long = 37.75# x 2 pieces	= 75.50#
03. Carbon Steel plate 48" x 84" x .25"	= 285.88#
04. Channel C8 x 11.5# x 48" long = 46# x 2 pieces	= 92.00#
05. Channel C4 x 5.4# x 88.5" long = 39.83# x 2 pieces	= 79.66#
06. Channel C4 x 5.4# x 56" long	= 25.20#
07. Channel C4 x 5.4# x 51.66" long	= 23.25#
08. Channel C4 x 5.4# x 24" long	= 10.80#
09. Unustrut channel P-1000 x 43.63 " long = 6.9# x 2 pieces	= 13.80#
10. Stiffening Plate 11.42" x 4" x .5" = 6.4# x 2 pieces	= 12.80#
11. Stiffening Plate 11.42" x 4" x .5" = 6.4# x 2 pieces	= 12.80#
12. Lifting Plate 6" x 3.5" x .5" = 2.95# x 2 pieces	= 5.90#
13. Channel C8 x 11.5# x 23" long = 22.04# x 2 pieces	= 44.08#

Total Weight = 835.00#

LIFTING FIXTURE WELDMENT WEIGHT AS PER  
DRAWING # 8875-MD-406223

01. Vertical Channel C4 x 5.4# x 88.5" = 39.83# x 2 pieces	= 79.66#
02. Lifting Plate 5.43" x 4" x .5" = 3.05# x 4 pieces	= 12.20#
03. Channel C6 x 10.5# x 65.75" long	= 57.53#
04. Rear Channel C6 x 10.5# x 68.75 long	= 57.53#
05. Channel C5 x 6.7# x 51.168" + 28.57# x 2 pieces	= 57.14#
06. Corner Gusset eyebolt plate 7.5" sq. x .75" thick x .281= 8.36# x 4	= 33.44#

Total Weight = 301.00#

Skid and Lifting Fixture combined total weights joined together =1136.00#

Note:

The above listed weights and a solid model of the upper and lower assemblies were completed to find Center of Gravity of unloaded skid for purposes of presumed load testing. Each assembly half was neither symmetrical or balanced nor was the Center of Gravity of the joined assemblies below the pick point. Ultimately I planned to make a solid model of both halves loaded. Dave Pushka stopped me at this point and after some conversation I recommended that perhaps this joined assembly should have multiple pick points to eliminate the need to find the CG as long as near center of lifting fixture, he agreed.

### SURGE TANK

ASME Head 52# x 2 pieces	= 104#
Rolled Cylinder Plate 39" x 75.39" x .25"	= 207#
Total Weight	= 311#

MOTOR/PUMP COMBINATION 158# each x 2

Total Weight = 316#

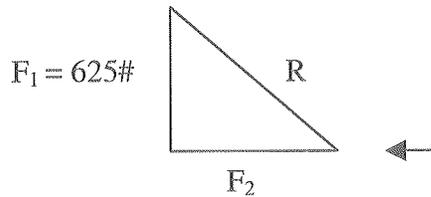
The major components on the cart are the Tank and Pumps which are located on diagonally opposite corners causing an initial weight balance of the Skid components not giving consideration to piping etc. Above listed major component weights cancel. Estimated dry weight including Skid and components is 2,500# as per drawing #8875.117-ME-406639, which indicates remainder of piping components weighs 737#.

After redesigning lifting fixture using tubular structural members and giving consideration to avoiding interferences with skid components, I was asked by Dave to investigate using as many pieces from the original design as was possible. The following states what could be used and commentary on loading.

Retained as many of lifting fixture components as possible going from single pick point to picking on all four corners. Items removed are 4" x 4" x 3/8" main lifting tube as well as two 1 1/4" x 1 1/4" x 3/16" cross tubes. Items retained are two C6 x 10.5# horizontal channels and two C4 x 7.25# vertical channels. Items added, two C5 x 6.7# channels which join C6 channels together and corner lifting plates.

Utilizing four pick points at corners, channel structural members are only seeing relatively small compression loads, 2500# total vertical load / 4 = 625# =  $F_1$

C6 x 10.5# x 60" channel description,  $A = 3.09 \text{ in}^2$ ,  $I = 0.866$ ,  $r = 0.529$  in weak direction and an unsupported length of 60" with a pick angle of  $60^\circ$  to horizontal.



$$R = F_1 / \sin\theta = 625 / \sin 60^\circ = 722\#$$

$$F_2 = R * \cos\theta = 722 * \cos 60^\circ = 361\# , \text{ or } 722\# \text{ load on channel}$$

Straight compressive load is  $722\# / 3.09$  or 234 psi.

Slenderness ratio is  $Kl / r = 1*60 / 0.529$  or 113.

AISC 1.5.1.3 Compression

$$\begin{aligned} F_a &= (1 - ((Kl/r)^2 / 2C_c^2) * F_y / 5/3 + (3(Kl/r) / 8) * C_c) - ((Kl/r)^3 / 8C_c^3) \\ &= (1 - (113^2 / 2 * 126^2)) * 36000 / 1.667 + 0.336 - 0.09 \\ &= 11254 \text{ psi or} \end{aligned}$$

$$F_a = F_y / 3$$

$$= 12000 \text{ psi which ever is less}$$

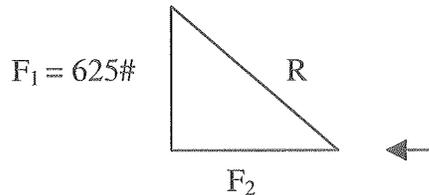
Where  $Kl/r$  of any unbraced section is less then  $C_c = \sqrt{(2\pi^2 E) / F_y}$

$$= \sqrt{(2\pi^2 * 29000000) / 36000}$$

$$= 126.099'' \text{ greater then } 113''$$

Utilizing same pick points as above

C5 x 6.7# x 51 5/16" channel description,  $A = 1.97 \text{ in}^2$ ,  $I = 0.479$ ,  $r = 0.493$  in weak direction and an unsupported length of 51" with a pick angle of  $65.5^\circ$  to horizontal.



$$R = F_1 / \sin\theta = 625 / \sin 60^\circ = 722\#$$

$$F_2 = R * \cos\theta = 722 * \cos 60^\circ = 361\# , \text{ or } 722\# \text{ load on channel}$$

Straight compressive load is  $722\# / 1.97$  or  $367 \text{ psi}$ .

Slenderness ratio is  $Kl / r = 1 * 52 / 0.493$  or  $105$

AISC 1.5.1.3 Compression

$$F_a = (1 - ((Kl/r)^2 / 2C_c^2)) * F_y / 5/3 + (3(Kl/r) / 8 * C_c) - ((Kl/r)^3 / 8C_c^3) *$$

$$= (1 - (105^2 / 2 * 126^2)) * 36000 / 1.667 + 0.313 - 0.072$$

$$= 12321 \text{ psi or}$$

$$F_a = F_y / 3$$

$$= 36000 / 3$$

$$= 12000 \text{ psi which ever is less}$$

Where  $Kl/r$  of any unbraced section is less than  $C_c = \sqrt{(2\pi^2 E) / F_y}$

$$= \sqrt{(2 \pi^2 * 29000000) / 36000}$$

$$= 126.099" \text{ greater than } 105"$$

Swivel hoist rings should be utilized, each having the capability to carry half the load or  $2500\#/2 = 1250\#$ . Nearest larger capacity hoist ring capacity is 2500# and requires 1/2" tapped hole at four corners as specified on drawings. Safety factor of two above working load.

With a 2500# total load / 4 corners or 625# per corner, look at weld size and stresses joining the 5" and 6" channels. Maximum linear inches of weld possible in the flange area joining two channels is 1.5" both sides of each flange \* 2 flanges, or  $6" * (.25 * \sin 45) = 1.06 \text{ in}^2$  of weld, with the maximum web weld length being 4" both sides, or  $8" * (.125 * \sin 45) = .707 \text{ in}^2$ . Stress in .25" and .125" welds are  $625/1.06 = 590 \text{ psi}$  and  $625/0.707 = 884 \text{ psi}$  respectively. E-70 fillet weld allowable of 11,200 psi, half of these welds are groove welds with an allowable of 15,800 psi.

Attachment of drilled/tapped lifting plates via welds to channels at corners. Top surface 3" weld at 3 corners of plate or  $9" * (.25 * \sin 45) = 1.59 \text{ in}^2$  of weld. Bottom surface in worst case allows for 3.5" weld or  $3.5" * (.25" * \sin 45) = 0.619 \text{ in}^2$  of weld. Stresses in welds are  $625/1.59 = 393 \text{ psi}$  and  $625/0.619 = 1010 \text{ psi}$  respectively. See attached drawing, all weld stresses are negligible.

Force on bolts joining upper lifting fixture to vertical channels of skid, 2500# total load/ 4 connections = 625# each connection / 2 bolts = 312.5 # load per bolt.  $P / A = 625\# / (2 * (\pi * 0.4^2 / 4)) = 2490 \text{ psi}$  which is < Fv 17,500 psi allowable for A325 bolts or < Fv 10,000 psi for A307 bolts as per AISC allowable in shear. Fy for A-325 bolts is 92 ksi Fy for A307 bolts not listed. 1592 psi < 17,500 psi or 10,000 psi. Stresses are negligible.